

FIRST-ORDER STRESSES AND DEFORMATIONS IN GLACIERS AND ICE SHEETS

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ABSTRACT. In this article the distribution of stress and velocities in glaciers and ice sheets is reinvestigated. We first derive the general equations governing non-linear viscous flow under plane deformations and formulate the relevant boundary conditions, including, in particular, a proper treatment of the accumulation–ablation mechanism. It is then shown how the emerging set of non-linear equations for the established boundary-value problem can be separated into a system covering steady-state problems on the one hand, and transient, time-dependent processes on the other hand. This separation is performed under the assumption that steady-state stresses are larger than the corresponding transient counterparts, suggesting a linearization of the transient equations with regard to the stresses. The steady-state equations are then analysed for the special case of an infinitely long, nearly parallel-sided slab. With the assumption that bottom undulations are small as compared to the glacier thickness it is shown that the original non-linear boundary-value problem can be decomposed into an infinite hierarchy of boundary-value problems defined on the simpler domain of the exactly parallel-sided slab, all of which are linear except for the lowest order one. Since its solution is readily available, the determination of the velocities and stresses due to bedrock protuberances is basically a linear problem, even though the constitutive response may be non-linear.

Assuming harmonic bedrock undulations we show for a Navier–Stokes fluid that the transfer of the bedrock undulations to the surface strongly depends on the mean inclination of the slab, but, more importantly, does now show a maximum when plotted as a function of wavelength λ . This result is contradictory to the corresponding results of Budd (1970[a]) and implies serious drawbacks to his calculations of longitudinal stresses and strain-rates in his subsequent article (Budd, 1970[b]). Yet, it is not true that for maximal transfer of bottom protuberances to the surface a distinct wavelength would not exist. The calculations of Budd must rather be extended to include non-linear constitutive behaviour, variations of temperature with depth, and sliding at the bed. It then turns out that under certain circumstances maximal transfer of bottom undulations to the surface in a distinct wavelength domain ($3 < \lambda < 5$) may indeed exist. Sliding at the bed and vertical temperature variation thereby play a decisive role.

Equally important is the stress distribution at the base, in particular the influence of the longitudinal strain effects on the latter. Rheological non-linearities, vertical temperature variations, and the sliding law at the bed play an important role and are investigated in detail.

For non-linear constitutive behaviour and spatially dependent temperature-variation solutions must be sought numerically. The finite-difference scheme used suggests a generalization of Glen's flow law so as to account for a nearly linear behaviour at low strain-rates.

We conclude with a perspective of possible extensions of the general theory to various other time-dependent and time-independent problems.

RÉSUMÉ. *La distribution des vitesses et des tensions de l'ordre supérieur dans les glaciers et les calottes de glace.* Le présent article est une réinvestigation de la distribution des vitesses et des tensions dans les glaciers et les calottes de glace. Nous établissons d'abord les équations générales du mouvement visqueux non linéaire dans les cas de déformations planes et formulons les conditions aux limites en tenant compte des phénomènes d'accumulation et d'ablation. Nous montrons ensuite que le système d'équations différentielles non linéaires obtenu peut être dissocié d'une part en problèmes stationnaires, d'autre part en problèmes transitoires. Cette dissociation suppose que les tensions stationnaires prévalent suffisamment sur les tensions transitoires, ce qui permet une linéarisation des équations transitoires relatives aux tensions. Les équations de l'état stationnaire sont ensuite examinées dans le cas spécial d'une couche presque parallèle et infiniment longue. Dans l'hypothèse où les inégalités du sol sont petites par rapport à l'épaisseur du glacier, nous montrons que le problème non linéaire des conditions aux limites peut être divisé en une hiérarchie infinie de problèmes, qui eux sont définis dans le domaine plus simple de la couche exactement parallèle et qui sont tous linéaires, à l'exception de celui du plus bas ordre. La solution de ce dernier étant facile à trouver, la détermination des vitesses et des tensions dues aux inégalités du sol est ramenée à un problème linéaire, bien que les lois de la mécanique des matériaux ne soient pas nécessairement linéaires.

Dans le cas d'ondulations sinusoidales du sol, nous montrons que, pour un liquide Navier–Stokes, la transmission de ces ondulations à la surface du glacier dépend fortement de l'inclinaison moyenne de la couche, mais aussi, et ceci est plus important, que la fonction de transfert dépendant de l'écartement des

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ondulations, n'a pas de maximum. Ce résultat contredit nettement les résultats de Budd (1970[a]) et suscite des doutes en ce qui concerne ses calculs de tensions et de vitesses de déformation (Budd, 1970[b]). Ceci ne veut pas dire qu'une longueur d'onde déterminée n'existerait pas pour un transfert maximum des inégalités du sol à la surface. Les calculs de Budd devraient plutôt être développés afin d'y inclure les lois de la mécanique non linéaire des matériaux, les variations de température et le glissement au sol. Il en découle que dans certaines conditions un transfert maximum des inégalités du sol à la surface peut exister en effet pour une certaine longueur d'onde ($3 < \lambda < 5$). Le glissement au sol et la variation verticale de la température y jouent cependant un rôle décisif.

La même importance revient à la distribution des tensions au sol, en particulier l'influence des déformations longitudinales sur ce dernier. Un comportement rhéologique non linéaire, les variations verticales de la température et la loi de glissement au sol jouent un rôle important et sont analysés en détail.

Pour un comportement non linéaire de la matière et une variation de température dépendent du lieu les solutions doivent être trouvées d'une manière numérique. A cause du schéma des différences finies employé dans ce travail il était cependant nécessaire de généraliser la loi de Glen et de tenir compte du comportement linéaire des matériaux sous faible tension.

En fin d'article nous donnons une vue du développement de la théorie générale appliquée à différents autres problèmes comportant ou ne comportant pas la variable temps.

ZUSAMMENFASSUNG. *Spannungen und Geschwindigkeiten höherer Ordnung in Gletschern und Eisschilden.* In der vorliegenden Arbeit wird die Geschwindigkeits- und Spannungsverteilung in Gletschern und Eisplatten einer neuerlichen Überprüfung unterzogen. Zuerst werden die allgemeinen Gleichungen für nichtlineares zähes Fliessen unter ebenen Deformationen hergeleitet sowie die relevanten Randbedingungen einschliesslich der Akkumulation-Ablation formuliert. Es wird dann gezeigt, wie das daraus hervorgehende System von nichtlinearen Differentialgleichungen für das aufgestellte Randwertproblem aufgespalten werden kann in stationäre Probleme einerseits und zum anderen in die transienten Probleme zeitabhängiger Prozesse. Diese Aufspaltung wird unter der Voraussetzung vorgenommen, dass die stationären Spannungsanteile die transienten hinreichend überwiegen, was eine Linearisierung der transienten Feldgleichungen bezüglich der Spannungen ermöglicht und nahelegt. Die Gleichungen des stationären Zustandes werden dann für den Spezialfall einer fast parallelen unendlich langen Schicht untersucht. Unter der Annahme dass die Bodenunebenheiten klein sind im Vergleich zur Dicke des Gletschers wird gezeigt, dass das nichtlineare Randwertproblem in eine unendliche Hierarchie von Randwertproblemen aufgespalten werden kann, die ihrerseits auf dem einfacheren Bereich der exakt parallelen Schicht definiert sind und alle mit Ausnahme der niedrigsten Ordnung linear sind. Da dessen Lösung leicht bestimmt werden kann, wird die Bestimmung der Geschwindigkeiten und Spannungen zufolge der Bodenunebenheiten im Wesen auf ein lineares Problem zurückgeführt, obwohl die Materialgesetze auch nicht-linear sein können.

Unter der Annahme harmonischer Bodenwellen wird für eine Navier-Stokes-Flüssigkeit gezeigt, dass die Übertragung der Bodenwellen auf die Oberfläche stark von der mittleren Neigung der Schicht abhängt, aber, noch wichtiger, dass die Transferfunktion in Abhängigkeit von der Wellenlänge kein Maximum aufweist. Dieses Resultat steht im Widerspruch zu entsprechenden Resultaten von Budd (1970[a]) und erweckt Zweifel an seinen Berechnungen von Spannungen und Verzerrungsgeschwindigkeiten in seiner Arbeit (Budd, 1970[b]). Es ist jedoch nicht so, dass für maximalen Transfer von Bodenunebenheiten auf die Oberfläche keine ausgezeichnete Wellenlänge existierte. Vielmehr sind die Buddschen Rechnungen auf nichtlineare Stoffgleichungen, Berücksichtigung der Temperaturvariationen und Gleiten am Bett zu erweitern. Es zeigt sich dann, dass unter bestimmten Voraussetzungen in der Tat maximale Übertragung von Bodenunebenheiten auf die Oberfläche für einen bestimmten Wellenlängenbereich ($3 < \lambda < 5$) möglich ist. Gleiten an der Sohle und vertikale Temperaturvariation spielen dabei eine entscheidende Rolle.

Von ebensolcher Wichtigkeit ist auch die Spannungsverteilung an der Sohle, insbesondere der Einfluss longitudinaler Verzerrungseffekte auf die letztere. Rheologische Nichtlinearitäten, Temperaturverteilung und das Gleitgesetz der Sohle spielen dabei eine wichtige Rolle und werden im Detail untersucht.

Für nichtlineares Materialverhalten und ortsabhängige Temperaturverteilung müssen die Lösungen auf numerischem Wege gesucht werden. Es war wegen des verwendeten Differenzenschemas aber nötig das Glen'sche Gesetz zu verallgemeinern und das lineare Stoffverhalten bei kleinen Spannungen zu berücksichtigen.

Wir schliessen mit einer Perspektive zur Ausdehnung der allgemeinen Theorie auf verschiedene andere sowohl zeitabhängiger wie zeitunabhängiger Probleme.

1. INTRODUCTION

The distribution of stress and velocities in glaciers and ice sheets has been treated previously by several authors. The effect of longitudinal strain on the state of stress at the base of a glacier or ice sheet was studied by Robin (1967) and Budd (1969) and has found its theoretical basis in the articles of Collins (1968), Nye (1969), and Budd (1970[a]). In these articles attention is focused on the significance of the longitudinal stresses set up by the flow of ice over protuberances on the bed. The idea is to improve the basal shear-stress formula, which, to

zeroth order, is independent of the material behaviour of ice and only involves the local glacier depth and the local inclination of its surface.

Steady-state stress and surface variations in infinite ice slabs with small undulations of their beds have equally been treated, notably by Yosida (1964) and Budd (1970[b]) who also gives further information on the pertinent literature. Yosida considered Newtonian viscous flow with no slip of a medium of uniform thickness down a uniform slope with small harmonic undulations superimposed on it. Budd generalized this solution by allowing perfect slip along the bed and by using an approximate solution technique to account for a non-linear viscous behaviour. His method gave answers to the question of how small bedrock irregularities are transferred to the surface topography and in what way they influence the stress distribution. In particular Budd (1970[b]) was able to give precise statements regarding the significance of the longitudinal stresses set up by the flow. He could explain under what circumstances the basal shear stress fluctuates in sympathy with the surface slope.

Budd's careful analysis requires the knowledge of a mean velocity across the depth of the ice slab. More precisely, in his work the velocity boundary conditions are determined by the shape of the base contour, the mean down-slope velocity, and the equation of continuity formulated as an integrated mass balance. This equation contains the accumulation rate, so that, in principle, the effect of the accumulation rate on the topography could be determined. Moreover, for a time-dependent accumulation rate, one would expect the surface undulations to be a direct result of the accumulation rate rather than the bedrock topography. The corresponding time-dependence of the surface topography will, under certain circumstances, result in surface waves travelling down the glacier akin to those treated by Nye (1960, 1963[a], [b]) in his kinematic-wave theory.

It is possible, if one so desires, to challenge the above work by, first, extending the analysis to include steady-state and transient accumulation rates and, secondly, to improve it by replacing the velocity boundary condition by a kinematic condition at the top surface. For it is quite clear that accumulation and ablation should enter the problem through a condition regarding the time-evolution of the glacier surface rather than an integrated mass balance. Moreover, the boundary condition at the ice-rock interface treated by Yosida as a no-slip condition and by Budd as a perfect slip could be generalized to account for velocity dependent friction. In other words, the determination of stress and velocities in glaciers and ice sheets and the propagation of surface waves should be unified and be derivable from a single general concept.

There exists a rational approach which delivers this unification, and explicit calculations can relatively easily be performed for an infinitely long slab with almost parallel sides. The method roughly proceeds as follows: As a first step, the general governing equations consisting of the balance laws of mass and momentum, the constitutive relations (Glen's flow law in Nye's three-dimensional extension), and the boundary conditions are derived. The latter include a statement about the sliding mechanism at the base, they express the stress tensor at the top surface in terms of the atmospheric conditions and involve a statement about the rate of change with time of the surface geometry in terms of the accumulation rate. These equations are perfectly general and no assumptions need to be invoked regarding the geometry of the ice sheet. The solution of a boundary-value problem is rather complex at this generality, however, due to two circumstances, first, Glen's law is non-linear, and, secondly, the geometry of the top surface is part of the solution of the boundary-value problem to be solved. The second step in attacking the complex problem is thus a restriction to simple geometries and simple flow configurations, or at least to patterns close to such situations. A realistic model for glaciers and ice sheets is the parallel-sided infinite slab of which the flow pattern and stress distribution was determined by Nye (1953, 1957). Problems, to which its steady-state behaviour can be considered as a first approximation can, among others, be formulated as follows:

- (i) What is the influence of small bedrock undulations on the stress and velocity distribution and on the surface topography?
- (ii) How does steady-state position-dependent accumulation rate influence the stress distribution and the surface geometry and, can this effect be separated from that of the bedrock undulations?
- (iii) What are the characteristic features of surface waves? In other words, does the kinematic wave theory suffice for the description of these waves, or does it have to be extended?
- (iv) Would such an extended wave theory perhaps give rise to a possible explanation of seasonal and surge-type waves?

Of these questions only question (i) has been sufficiently treated; no systematic study of the effect of steady accumulation rate is known to us, and the transient response is uninvestigated except for some remarks made by Budd (1970[b]). Moreover, since Budd's solution is based on an approximate integration technique, and because he does not seem to treat the influence of the inclination of the ice slab and of the sliding mechanism systematically, it is justified to reinvestigate item (i) at least from this restricted point of view.

The aim here is an attempt to provide answers to the above-stated questions. In the present paper we shall derive the basic governing equations, but shall deal in detail only with the steady-state response. The transient behaviour and, in particular, surface wave problems are reserved to another paper (Hutter, 1980) as is a direct approach to the longitudinal strain effects on basal shear (Hutter, 1981). Hence, only items (i) and (ii) above will be treated here. Attention will exclusively be focused on the nearly-parallel-sided slab. This restriction is tantamount to the neglect of effects of accumulation on the geometry of the ice slab, which therefore limits consideration to length scales which are not asymptotically large as compared to glacier thicknesses. The results of the present paper, however, are meaningful on length scales over which ice thicknesses do not change considerably.

The rationale in achieving an analysis of the nearly-parallel-sided slab is, first to establish field equations and boundary conditions for stress and velocity in a suitably non-dimensional form, secondly to replace the boundary-value problem as stated for the true geometry by an approximate one of the strictly parallel-sided slab and, thirdly to solve this approximate boundary-value problem by a regular perturbation approach. The non-dimensionalization is different from the usual ones in liquid-film theories; contrary to the latter, it is valid for all inclinations γ of the slab, including $\gamma = 0$. In the application of the perturbation procedure we shall, however, restrict considerations to values of γ which are bounded away from zero, but nevertheless indicate how calculations should proceed for small γ or $\gamma = 0$.

2. BASIC GOVERNING EQUATIONS

The model of a glacier or ice sheet is depicted in Figure 1. It shows a vertical cut through an infinitely wide glacier; the upper surface and bed are thus cylindrical. In the mean these surfaces are assumed to be plane over most parts of the glacier. Apart from the glacier head and the region at the snout, the bottom and top surfaces are thus approximately parallel, or for a distance that is large compared with the ice thickness may be regarded as approximately parallel. The Cartesian coordinate system is taken such that the x -axis is parallel to the mean direction of the top or bottom surface and the local depth is measured perpendicular to the x -axis. Both top and bottom surface are inclined with respect to the x -direction so that, in general α and β do not vanish. Yet, we shall assume that the deviation of the glacier surface from its mean $y = D$ and of the bottom topography from the line $y = 0$ are small. Denoting the distances of these surfaces from the x -axis by y_s and y_b , respectively, we thus may assume $|y_b|$ and $|y_s - D|$ to be small in comparison with the glacier thickness.

We restrict considerations to plane strain and presume the ice to be isotropic, but generally we shall not strictly assume it to obey Nye's extension of Glen's flow law. The reason is the singular behaviour of Glen's flow law at small stress deviators, which leads in certain problems to mathematical singularities, which can be removed by a realistically modified constitutive relationship. As for Glen's flow law this constitutive relationship is an incompressible non-linear viscous fluid model. We do not presume that temperature effects are negligible, but allow for a variation of temperatures perpendicular to the main flow. The glacier may thus be cold or temperate. For most of the analysis the latter will be assumed.

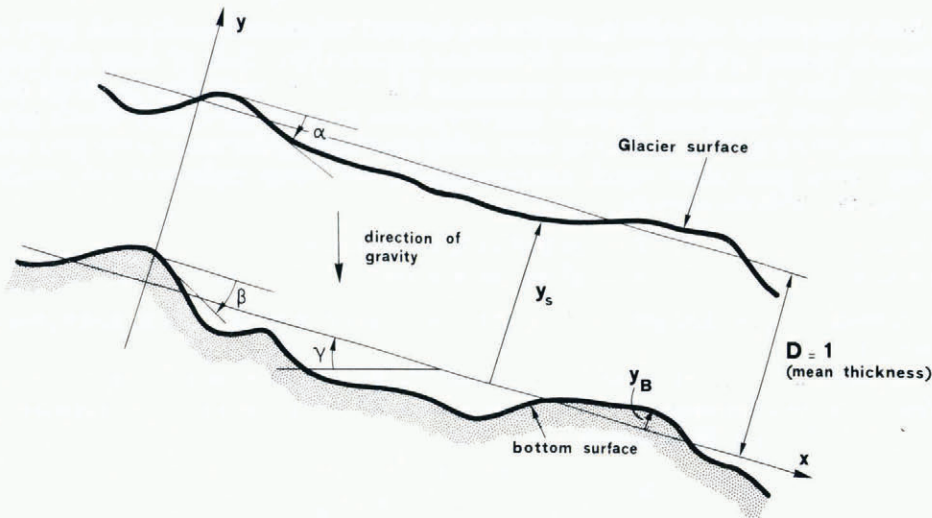


Fig. 1. Nearly parallel-sided slab. Definition of geometry.

2.1. On an extension of Glen's flow law

Glen's power law carries the disadvantage that stress deviators grow infinitely fast at small stretchings. In some of the numerical solutions to be presented below this turns out to lead to singular behaviour. Rather than deal with these singularities mathematically, we shall, when necessary, replace Glen's flow law by another one that does not show this singular behaviour. The idea is not new and has independently been presented by Hutter (1979) and Thompson (1979). To introduce this more general constitutive relation, let \mathbf{d} be the stretching tensor and \mathbf{t}' the stress deviator. The class of constitutive relationships given by

$$\mathbf{d} = AB(t_{II}^2)^n \mathbf{t}', \quad (1)$$

in which A and n are constants and B is a function of the second stress deviator invariant $t_{II}^2 = \frac{1}{2} \mathbf{t}' : \mathbf{t}'$, embraces Glen's flow law as well as its extension

$$B = (t_{II}^2)^{(n-1)/2} + k. \quad (2)$$

The constant k introduces a nearly linear Newtonian behaviour at small stretchings and removes the singularity mentioned above. For brevity we shall call Equation (1) with the specification (2) the *generalized Glen flow law*. A and n are known from the usual law and k can

be determined from creep experiments at low strain-rate. Glen's flow law is obtained for $k \equiv 0$.*

Below we shall introduce a non-dimensionalized version of the function B , denoted by \mathfrak{B} and defined by

$$\mathfrak{B}(\tau \Pi^2) \equiv \frac{B(t \Pi^2)}{B(\rho^2 g^2 D^2)} \equiv \frac{B(\rho^2 g^2 D^2 \tau^2)}{B(\rho^2 g^2 D^2)}.$$

For the generalized Glen flow law this function reads

$$\mathfrak{B}(x^2) = \frac{(x^2)^{(n-1)/2 + \mathfrak{k}}}{1 + \mathfrak{k}}, \quad \text{with } \mathfrak{k} = \frac{k}{(\rho g D)^{n-1}}.$$

To find a numerical value for \mathfrak{k} consider the material responses $Ak = \dot{\epsilon}/\sigma$ and $A\sigma^{n-1} = \dot{\epsilon}/\sigma$ for a linearly viscous and a Glen-type behaviour respectively. Both of these are balanced if $k = \sigma^{n-1}$. Linear behaviour is more important if $k > \sigma^{n-1}$. According to recent work of Duval, Budd, and Thompson (private communication from L. Liboutry) Glen's flow law is valid down to 2×10^4 Pa, so that we may safely assume that the linear stress part and the non-linear stress part share equal contributions in the stress-strain-rate relationship at roughly 10^4 Pa. This then yields

$$\begin{aligned} \text{for } n = 3, & & k = 10^4 \text{ Pa}^2 & \rightarrow \mathfrak{k} \approx 10^{-3} - 10^{-4}, \\ \text{for } n = 2, & & k = 10^8 \text{ Pa} & \rightarrow \mathfrak{k} \approx 10^{-2}. \end{aligned}$$

In the subsequent calculations we shall therefore choose $n = (2, 3)$ and correspondingly $\mathfrak{k} = (10^{-2}, 10^{-3})$.

In cold ice the temperature varies very nearly with a distinct coordinate, say y ; such a dependence can be approximately taken into account by assuming A to be y -dependent. For such a case the function \mathfrak{B} is defined as

$$\mathfrak{B}(\tau \Pi, y) \equiv \frac{A(y)}{A(y_0)} \frac{B(\rho^2 g^2 D^2 \tau \Pi^2)}{B(\rho^2 g^2 D^2)}.$$

$A(y)$ is obtained, if in the Arrhenius law $A = \mathfrak{A} \exp(-Q/(kT))$, in which \mathfrak{A} is a constant, Q the activation energy, k the Boltzmann constant, and T the absolute temperature, the latter is substituted as a function of y , y_0 is a reference point, usually the surface.

Before proceeding a word of caution seems to be in order. In what follows, perturbation solutions for small bedrock undulations with amplitude ϵ will be sought, and these perturbation solutions will be constructed considering $\epsilon \rightarrow 0$, keeping \mathfrak{k} fixed and finite. In fact, were we to consider also the limit $\mathfrak{k} \rightarrow 0$, our numerical solution technique would result in mathematical singularities (at the free surface). Clearly, a solution procedure for which both limits $\epsilon \rightarrow 0$ and $\mathfrak{k} \rightarrow 0$ could be pursued, would be more appropriate. We shall not proceed along these lines, but the reader should be aware that some of our results might become invalid, whereas they would still be valid were the proper asymptotic analysis to be undertaken. We shall explicitly point out where this happens.

2.2. Field equations

In order to simplify the subsequent analysis, the governing field equations will be written in dimensionless form. To this end a characteristic length D and a characteristic time τ will be introduced. Stresses are then non-dimensionalized with the reference stress $\rho g D$ and velocities with $V = D/\tau$. With these scales and with the plane-strain assumption, the balance laws of mass and momentum and the constitutive relations assume the form

* Incidentally, Equation (2) has also been introduced from a material-scientist point of view, see Colbeck and Evans (1973).

$$\left. \begin{aligned}
 \frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} &= 0, \\
 \frac{\partial \sigma_x}{\partial x} + \frac{\partial \tau}{\partial y} + \sin \gamma &= R_x = \mathcal{F} \left\{ \frac{\partial u}{\partial t} + u \frac{\partial u}{\partial x} + v \frac{\partial u}{\partial y} \right\}, \\
 \frac{\partial \tau}{\partial x} + \frac{\partial \sigma_y}{\partial y} - \cos \gamma &= R_y = \mathcal{F} \left\{ \frac{\partial v}{\partial t} + u \frac{\partial v}{\partial x} + v \frac{\partial v}{\partial y} \right\}, \\
 \frac{\partial u}{\partial x} &= \frac{1}{2} \mathcal{G} \mathcal{B} (\tau_{II}^2) (\sigma_x - \sigma_y) = -\frac{\partial v}{\partial y}, \\
 \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} &= 2 \mathcal{G} \mathcal{B} (\tau_{II}^2) \tau,
 \end{aligned} \right\} \tag{3}$$

where an explicit dependence of \mathcal{B} on y is omitted and where

$$\tau_{II}^2 = \frac{1}{4} (\sigma_x - \sigma_y)^2 + \tau^2,$$

is the second invariant of the stress deviator. Moreover, \mathcal{F} and \mathcal{G} are a dimensionless Froude and a ‘‘Glen’’ number,

$$\mathcal{F} = \frac{U^2}{gD} = \frac{D}{g\tau^2}, \quad \mathcal{G} = \tau A(y_0) \rho g D f (\rho^2 g^2 D^2), \tag{4}$$

whose values depend on the selection of D and τ (or U). Choosing for D the mean glacier thickness guarantees that the dimensionless stresses vary between -1 and 1 . It is then straightforward to see that \mathcal{F} is very small ($< 10^{-6}$) for even unrealistically large values of the velocity U . This justifies the neglect of the local and convective acceleration terms and allows for a relatively free choice of the characteristic velocity U or time τ . Three different choices are directly suggested by the problem, namely,

- (i) to simply assign a specific value to \mathcal{G} ($= 1$);
- (ii) to choose U according to realistic longitudinal surface velocities;
- (iii) to scale velocities with the accumulation rate.

The first choice is mathematically convenient, the second and third are physically suggested. For case (ii) emphasis is laid on a proper treatment of velocity profiles and accumulation is regarded as a secondary effect. In the third case, accumulation is regarded as important. Realistic values for surface velocities and accumulation rate are, perhaps, 100 m a^{-1} and 1 m a^{-1} , respectively. If we then choose $D = 100$, $a = 1 \text{ m/a}$, $n = 3$, $A = 10^{-19}$

TABLE I. ORDERS OF MAGNITUDES FOR VARIOUS CHOICES OF THE CHARACTERISTIC VELOCITY U OR TIME τ

	U m/a	τ a	\mathcal{G}	\mathcal{F}
(i)	10^4	10^{-2}	1	10^{-10}
(ii)	10^2	1	10^2	10^{-14}
(iii)	1	10^2	10^4	10^{-18}

$\text{dyn}^{-n} \text{cm}^{2n} \text{a}^{-1}$ the orders of magnitude shown in Table I are obtained. If one scales equations with the choice (ii) one guarantees that the dimensionless velocity u and the stresses (σ_x, σ_y) are of order one. The dimensional quantities are then obtained from the non-dimensional ones according to

$$(u, v)_{\text{dim}} = \frac{D}{\tau} (u, v), \quad (\sigma_x, \sigma_y, \tau, p)_{\text{dim}} = \rho g D (\sigma_x, \sigma_y, \tau, p), \quad (5)$$

and realistic values for these are

$$(u, v)_{\text{dim}} \approx (100-300, 1) \text{ m a}^{-1}, \quad (\sigma_x, \sigma_y, \tau, p)_{\text{dim}} \approx (10, 10, 1, 10) \text{ bar}. \quad (6)$$

In what follows Equations (3), which form a set of five partial differential equations for five unknowns, will be solved neglecting inertial terms ($\mathbf{f} \rightarrow 0$). The emerging system then contains only one dimensionless number, namely \mathcal{G} . It is possible, if one so desires, to absorb this constant in a new dimensionless velocity, by the transformation $(u, v) \rightarrow (u, v)/\mathcal{G}$, and it is readily seen that this simply corresponds to a non-dimensionalization of the equations according to entry (i) in the above table, needless to say one is not allowed in this case to assume that non-dimensional velocities are of order one. This is only a formal disadvantage because it is quite clear what physical solution one is trying to extract from the problem in hand. As far as computer adaptations of the equations are concerned, the imbalance in the orders of magnitude of dimensionless stresses and velocities is unlikely to cause round-off errors large enough to invalidate the numerical results.* Since moreover with the choice $\mathcal{G} = 1$ the formulae are free from an unnecessary constant and it is anyhow clear how to bring the dimensionless velocities back to order 1, we shall in the subsequent analysis use $\mathcal{G} = 1$ and thus restrict considerations to row (i) in Table I.

Frequently it is advantageous to work with the stress deviator rather than with stresses. Denoting the former by primes these are related by

$$\sigma_x' = \frac{1}{2}(\sigma_x - \sigma_y), \quad \sigma_y' = \frac{1}{2}(\sigma_y - \sigma_x). \quad (7)$$

Introducing the dimensionless pressure p as an unknown, the momentum equations read in this case

$$\left. \begin{aligned} \frac{\partial \sigma_x'}{\partial x} + \frac{\partial \tau}{\partial y} - \frac{\partial p}{\partial x} + \sin \gamma &= R_x, \\ \frac{\partial \tau}{\partial x} + \frac{\partial \sigma_y'}{\partial y} - \frac{\partial p}{\partial y} - \cos \gamma &= R_y. \end{aligned} \right\} \quad (8)$$

When written in terms of the stress deviators the field equations (3) comprise six equations for six unknowns, the new unknown being the pressure.

Finally, we mention that there are still other possibilities for the non-dimensionalizations. For instance, one may non-dimensionalize stress using a reference stress $\rho g D$ and may introduce a dimensionless pressure \bar{p} according to

$$p_{\text{dim}} = p_{\text{atm}} - \rho g D \cos \gamma (1 - \bar{y}) + \rho g \sin \gamma \bar{p}.$$

Denoting dimensionless quantities by overhead bars, it can be shown that the field equations (2) become in this case

$$\left. \begin{aligned} \frac{\partial \bar{u}}{\partial x} + \frac{\partial \bar{v}}{\partial y} &= 0, \\ \frac{\partial \bar{\sigma}_x'}{\partial x} + \frac{\partial \bar{\tau}}{\partial y} - \frac{\partial \bar{p}}{\partial x} + 1 &= 0, \quad \frac{\partial \bar{\tau}}{\partial x} + \frac{\partial \bar{\sigma}_y'}{\partial y} - \frac{\partial \bar{p}}{\partial y} = 0, \\ \frac{\partial \bar{u}}{\partial x} &= \overline{\mathcal{G}^2} (\bar{\tau}_{\text{II}}^2) \bar{\sigma}_x', \quad \frac{\partial \bar{v}}{\partial y} = \overline{\mathcal{G}^2} (\bar{\tau}_{\text{II}}^2) \bar{\sigma}_y', \\ \frac{\partial \bar{u}}{\partial y} + \frac{\partial \bar{v}}{\partial x} &= 2 \overline{\mathcal{G}^2} (\bar{\tau}_{\text{II}}^2) \bar{\tau}, \end{aligned} \right\} \quad (9)$$

* The word length of the computer used in the evaluation of the numerical results is 14 digits in the decimal system when single precision is used and 28 in double precision. According to the table on the preceding page, velocities vary only by two decimals in cases (i) and (ii).

where

$$\left. \begin{aligned} \bar{\tau}_{\Pi^2} &= \frac{1}{4}(\bar{\sigma}_x + \bar{\sigma}_y)^2 + \bar{\tau}^2, & \mathfrak{B}(\tau_{\Pi^2}) &= \frac{f(\rho^2 g^2 D^2 \sin^2 \gamma \bar{\tau}_{\Pi^2})}{f(\rho^2 g^2 D^2 \sin^2 \gamma)}, \\ \bar{\mathfrak{G}} &= \tau A \rho g D \sin \gamma f(\rho^2 g^2 D^2 \sin^2 \gamma). \end{aligned} \right\} \quad (10)$$

This is the non-dimensionalization used in the theory of viscous liquid films. The system of Equations (9) has the advantage of being better adjusted to the fact that the flow is gravity driven. Certain spurious singularities which occur for small γ with Equations (3) can be shown to be removable when using Equations (9) yet Equations (9) are definitely improper for $\gamma = 0$, whereas Equations (3) remain valid. The subtleties shall be pointed out at the appropriate places.

2.3. *Boundary conditions*

To complete the formulation of the problem, the field equations must be complemented by boundary conditions. *At the base* the latter will be expressed in either one of the following two ways. At first, we assume perfect slip with no cavity formation and thus may write

$$\left. \begin{aligned} u &= \bar{u}, \\ v &= \frac{dy_B}{dx} u, \end{aligned} \right\} \quad \text{at } y = y_B(x). \quad (11)$$

Here \bar{u} is assumed to be a prescribed sliding velocity, which as we shall see must be independent of time and space, see below. It is not the same as the tangential speed, but is proportional to it. For $\bar{u} = 0$ Equation (11) includes the no-slip condition.

There is another possibility of formulating the boundary conditions at the base of a glacier without cavity formation. In this case one sets

$$\left. \begin{aligned} v &= \frac{dy_B}{dx} u, \\ u_{||} &= \mathcal{C} \tau_{||}^m. \end{aligned} \right\} \quad \text{at } y = y_B(x). \quad (12)$$

The first of these is the tangency condition. The second, on the other hand expresses a velocity-dependent friction law akin to that of Weertman (1957). For $\mathcal{C} = 0$ it includes the no-slip condition. A proper regelation mechanism (Nye, 1970; Kamb, 1970) will not be treated here.

At the top surface we require that stress is continuous and that the conditions of a kinematic surface are satisfied. As regards the latter, it should be pointed out that the free surface is not a material surface, since due to accumulation and ablation particles are added or removed. If $F := \mathcal{Y}_s(x, t) - y = 0$ is the equation of the surface we must thus have

$$\frac{\partial \mathcal{Y}_s}{\partial t} + \frac{\partial \mathcal{Y}_s}{\partial x} u - v = a(x, t), \quad \text{at } y = \mathcal{Y}_s(x, t), \quad (13)$$

where $a(x, t)$ is the dimensionless accumulation rate, which is positive as an accumulation rate and negative as an ablation.

The boundary conditions of stress on the other hand read

$$\left. \begin{aligned} \sigma_x \sin^2 \alpha + \sigma_y \cos^2 \alpha + \tau \sin 2\alpha &= -p, \\ \tau - \frac{1}{2}(\sigma_y - \sigma_x) \tan 2\alpha &= 0, \end{aligned} \right\} \quad \text{at } y = \mathcal{Y}_s(x, t). \quad (14)$$

Here α is the surface inclination so that

$$\tan \alpha = -\frac{d\mathcal{Y}_s}{dx}. \quad (15)$$

The minus sign stems from the definition of positive angles α ; p is the dimensionless atmospheric pressure, which, in general, is a function of x and t . We shall neglect this variation and assume p to be constant.

With the inclusion of the boundary conditions at the base and at the top surface a well-set boundary-value problem has been obtained, in which $\mathcal{Y}_s(x, t)$ must also be considered as unknown. The equation for its determination is Equation (13).

Before proceeding further it is worthwhile to give an order of magnitude for the accumulation rate. For this purpose note that

$$a_{\text{dim}} = \frac{D}{\tau} a. \quad (16)$$

With $a_{\text{dim}} = 10 \text{ m a}^{-1}$, D and τ as before, a value of $a \leq 10^{-3}$ to 10^{-4} is obtained. Hence the dimensionless accumulation rate is very small, and to zeroth order it may safely be neglected. Clearly, these values are hinged to the choice of the characteristic time; it precludes analysis of accumulation effects on glacier geometry. Moreover, the friction constant in the first of Equations (12) is related to its dimensional counterpart by

$$\mathcal{C} = \mathcal{C}_{\text{dim}} \frac{\tau}{D} (\rho g D)^m, \quad (17)$$

and is an appropriately chosen constant.

2.4. Decoupling of the governing equations into steady-state and transient problems

One of the major questions raised in this article is the same as that asked and already answered to a high degree of satisfaction by Budd (1968, 1969, 1970[a], [b]) namely to evaluate the influence of the bedrock undulations on the surface topography. Another is to estimate the effect of the accumulation as a function of position or time or both. This second problem will lead to surface waves travelling down the glacier, akin to the kinematic waves studied by Nye (1960, 1963 [a], [b]).

Below, the separation of the total fields into steady-state and time-dependent fields will be demonstrated although ultimately in this paper only the steady-state problem will be treated, because this paper will serve as a reference for others (Hutter, 1980, 1981).

To separate the time-dependent from the time-independent problem, let

$$u = \hat{u} + \tilde{u}, \quad \text{etc.}, \quad (18)$$

be the decomposition of the total fields into steady-state (circumflexes) fields and quantities which are deviations from this state (tildes). By introducing the separations (18) in the field equations and boundary conditions, boundary-value problems for the steady-state and the purely transient part of the problem can be obtained. This decomposition being straightforward, we shall give below the resulting equations only.

(a) Steady-state problem

Formally, the steady-state equations are obtained from Equations (3), (11), (12), (13), and (14) by merely dropping local time derivatives and using circumflexes. This essentially amounts to formal changes in the kinematic boundary condition at the top surface, which now becomes

$$\frac{\partial \hat{\mathcal{Y}}_s}{\partial x} \hat{u} - \hat{v} = \hat{a}(x), \quad \text{at } y = \hat{\mathcal{Y}}_s(x), \quad (19)$$

and in appropriate changes of the momentum equations, if R_x and R_y should be kept.

For brevity the equations will not be repeated here.

(b) *Transient problem*

For the solution of the transient problem the solution to the steady problem is assumed to be known. In the transient problem we then only search for the deviations from the total fields. This process of separation is straightforward in principle, but somewhat messy if no resort is made to a simplifying assumption. We shall do this and *postulate that the steady-state stresses are much larger than the corrections due to a possible time-dependent effect*. Based on this assumption the constitutive relations in the fourth and fifth of Equations (3) can be linearized. This linearization in the stresses cannot be over-emphasized. In particular, it does not imply that \tilde{u} and \tilde{v} need be small. A linearization in these quantities might indeed be inappropriate.

The basic governing equations are:

$$\left. \begin{aligned} \frac{\partial \tilde{\sigma}_x}{\partial x} + \frac{\partial \tilde{\tau}}{\partial y} &= 0, & \frac{\partial \tilde{\tau}}{\partial x} + \frac{\partial \tilde{\sigma}_y}{\partial y} &= 0, \\ \frac{\partial \tilde{u}}{\partial x} + \frac{\partial \tilde{v}}{\partial y} &= 0, \\ \frac{\partial \tilde{u}}{\partial x} &= \frac{1}{2} \mathfrak{B}(\hat{\tau}_{II}^2)(\tilde{\sigma}_x - \tilde{\sigma}_y) + \frac{1}{2} \mathfrak{B}'(\hat{\tau}_{II}^2)(\tilde{\tau}_{II}^2)(\hat{\sigma}_x - \hat{\sigma}_y), \\ \frac{\partial \tilde{u}}{\partial y} + \frac{\partial \tilde{v}}{\partial x} &= 2 \mathfrak{B}(\hat{\tau}_{II}^2) \tilde{\tau} + 2 \mathfrak{B}'(\hat{\tau}_{II}^2)(\tilde{\tau}_{II}^2) \hat{\tau}, \end{aligned} \right\} \quad (20)$$

in which $\mathfrak{B}'(x) = d\mathfrak{B}/dx$ and

$$\left. \begin{aligned} \hat{\tau}_{II}^2 &= \frac{1}{4}(\hat{\sigma}_x - \hat{\sigma}_y)^2 + \hat{\tau}^2, \\ \tilde{\tau}_{II}^2 &= 2[\frac{1}{4}(\hat{\sigma}_x - \hat{\sigma}_y)(\tilde{\sigma}_x - \tilde{\sigma}_y) + \hat{\tau}\tilde{\tau}], \end{aligned} \right\} \quad (21)$$

are a steady-state and a transient second stress-deviator invariant. On the other hand, the boundary conditions become:

At the base:

$$\left. \begin{aligned} \tilde{u} &= \tilde{\tilde{u}}, & \tilde{u} &= \mathcal{C}m\hat{\tau}^{m-1}\tilde{\tau}, \\ v &= \frac{dy_B}{dx} \tilde{\tilde{u}}, & \tilde{v} &= \frac{dy_B}{dx} \tilde{\tilde{u}}, \end{aligned} \right\} \quad \text{at } y = y_B(x). \quad (22)$$

The formulae on the left apply when the boundary condition is of kinematic nature, those on the right when a velocity-dependent sliding condition is used; $\tilde{\tilde{u}}$ is assumed to be known.

At the top surface:

for the kinematic condition

$$\frac{\partial \hat{Y}_s}{\partial t} + \frac{\partial \hat{Y}_s}{\partial x} \tilde{u} + \frac{\partial \hat{Y}_s}{\partial x} \tilde{u} + \frac{\partial \hat{Y}_s}{\partial x} \tilde{u} - \tilde{v} = \tilde{a}, \quad \text{at } y = \hat{Y}_s(x) + \hat{Y}_s(x, t), \quad (23)$$

and for the boundary condition of stress

$$\left. \begin{aligned} (\hat{\sigma}_x + \tilde{\sigma}_x) \sin^2 \alpha + (\hat{\sigma}_y + \tilde{\sigma}_y) \cos^2 \alpha + (\hat{\tau} + \tilde{\tau}) \sin 2\alpha &= 0, \\ \hat{\tau} + \tilde{\tau} + \frac{1}{2}(\hat{\sigma}_x - \hat{\sigma}_y + \tilde{\sigma}_x - \tilde{\sigma}_y) \tan 2\alpha &= 0, \\ \text{at } y &= \hat{Y}_s(x) + \hat{Y}_s(x, t). \end{aligned} \right\} \quad (24)$$

In these equations α is the total angle of surface inclination, $\alpha = \hat{\alpha} + \tilde{\alpha}$, so that

$$\sin^2(\hat{\alpha} + \tilde{\alpha}) = (\sin \hat{\alpha} \cos \tilde{\alpha} + \cos \hat{\alpha} \sin \tilde{\alpha})^2, \quad \text{etc.}$$

Equations (24) complete the formulation of the problem. In the following sections a systematic approach will be shown which allows us to attack various problems occurring in the velocity and stress distribution of glaciers and large ice sheets.

3. STEADY-STATE SOLUTION FOR THE NEARLY PARALLEL-SIDED SLAB

The equations of the steady-state problem will now be subject to an approximate solution scheme. The key idea in this is the assumption that the glacier surface and the bottom topography do not deviate considerably from the infinite or semi-infinite parallel-sided slab. On this presumption an iterative scheme will be developed which allows us, first, to calculate the influence of the bottom topography on the surface geometry and, secondly, to estimate the influence of a steady accumulation or ablation rate on the stress and velocity distribution as well as on surface topography. Some of these questions have already been answered by Budd (1970[a], [b]), however his solution is approximate, and here we aim at a justification or disproof of his approach.

3.1. *Steady-state behaviour of an infinite slab on a bed with small undulations*

(a) *Perturbation scheme*

The fundamental assumption on which the following perturbation approach is based is that amplitudes of the base are small in comparison to the mean glacier thickness. Consequently, we may write

$$y_B(x) = \epsilon \Lambda(x) \quad \text{where } 0 < \epsilon \ll 1. \quad (25)$$

In this equation $\Lambda(x)$ and its derivative $\Lambda'(x)$ are $O(1)$. The representation (25) suggests searching for a perturbation solution in the form

$$(\hat{\sigma}_x, \hat{\sigma}_y, \hat{\tau}, \hat{u}, \hat{Y}_s) = \sum_{\nu=0}^{\infty} \epsilon^{\nu} (\sigma_x^{(\nu)}, \dots, Y_s^{(\nu)}), \quad \hat{v} = \sum_{\nu=1}^{\infty} \epsilon^{\nu} v, \quad (26)$$

in which $Y_s^{(0)}$ may be set equal to 1, since the depth is presumed determined from the solution of the flow problem over length scales much larger than this depth. In the second of Equations (26) it was further assumed that $v = O(\epsilon u)$, which is in agreement with observed speeds. Furthermore, lowest-order stresses are all assumed not to be small. As we shall see later on

τ will be proportional to $\sin \gamma$, implying that γ should not become small if the representations (26) are meaningful. It is obvious, however, how the perturbation approach can be rectified for small mean inclination angles γ without the introduction of the alternative non-dimensionalized equations (9). One simply sets $\tau^{(0)} = 0$ and starts the perturbation expansion for τ with first- (or even higher-) order terms. In view of the results to be presented in Section 4, this remark is important.

If the representations (26) are substituted into the field equations and boundary conditions of the steady-state problem and if, in the respective equations, terms of equal powers in ϵ are collected, a hierarchy of boundary value problems is obtained. For instance, the zeroth- and first-order field equations are as follows:

(i) zeroth-order field equations

$$\left. \begin{aligned} \frac{\partial \sigma_x^{(0)}}{\partial x} + \frac{\partial \tau^{(0)}}{\partial y} + \sin \gamma &= 0, & \frac{\partial \tau^{(0)}}{\partial x} + \frac{\partial \sigma_y^{(0)}}{\partial y} - \cos \gamma &= 0, \\ \frac{\partial u^{(0)}}{\partial x} &= 0, & 0 &= \frac{1}{2} \mathfrak{B}^{(0)}(\tau_{\Pi^2}^{(0)}) (\sigma_x - \sigma_y), & \frac{\partial u^{(0)}}{\partial y} &= 2 \mathfrak{B}^{(0)}(\tau_{\Pi^2}^{(0)}) \tau, \\ & & \tau_{\Pi^2}^{(0)} &= \frac{1}{4} (\sigma_x - \sigma_y)^2 + \tau^2, \end{aligned} \right\} \quad (27)$$

(ii) first-order field equations

$$\left. \begin{aligned} \frac{\partial \sigma_x^{(1)}}{\partial x} + \frac{\partial \tau^{(1)}}{\partial y} &= 0, & \frac{\partial \tau^{(1)}}{\partial x} + \frac{\partial \sigma_y^{(1)}}{\partial y} &= 0, \\ & & \frac{\partial u^{(1)}}{\partial x} + \frac{\partial v^{(1)}}{\partial y} &= 0, \\ \frac{\partial u^{(1)}}{\partial x} &= \frac{1}{2} \mathfrak{B}^{(0)}(\tau_{\Pi^2}^{(0)}) (\sigma_x - \sigma_y) + \frac{1}{2} \mathfrak{B}'^{(0)}(\tau_{\Pi^2}^{(0)}) (\tau_{\Pi^2}^{(0)}) (\sigma_x - \sigma_y), \\ \frac{\partial u^{(1)}}{\partial y} + \frac{\partial v^{(1)}}{\partial x} &= 2 \mathfrak{B}^{(0)}(\tau_{\Pi^2}^{(0)}) \tau + 2 \mathfrak{B}'^{(0)}(\tau_{\Pi^2}^{(0)}) (\tau_{\Pi^2}^{(0)}) \tau, \\ \tau_{\Pi^2}^{(1)} &= 2 \left[\frac{1}{4} (\sigma_x - \sigma_y) (\sigma_x - \sigma_y) + \tau \tau \right]. \end{aligned} \right\} \quad (28)$$

Higher-order equations could also easily be deduced. They are of little interest, however.

The derivation of the approximate boundary conditions is based on the idea that the condition valid on the boundary surface is replaced by an appropriate condition on the mean bed and mean surface respectively. To explain the procedure, consider Equation (11), in which \bar{u} and $y_B(x)$ must be regarded as known functions of x ; \hat{u} and \hat{v} , on the other hand are functions of the form

$$\hat{u} = \hat{u}(x, y_B(x)) = \hat{u}(x, \epsilon \Lambda(x)), \dots$$

Developing these functions in terms of Taylor series expansions of the second variable and substituting the resulting series into the equations results in new forms of the boundary conditions in which all functions are evaluated at $y = 0$, rather than at the actual bed. Hence we may say that the simple relations (11) on the complex geometry $y_B(x)$ are replaced by formally more complex equations on a much simpler geometry, namely $y = 0$. The complexity of the new equations is not really increased as compared to Equations (11), however, because the small parameter ϵ will enter and allow us to make use of the expansions (26). If this is done the *kinematic boundary conditions at the base* read, to first order,

$$\left. \begin{aligned} u &= \bar{u}, & u + \frac{\partial u}{\partial y} \Lambda &= 0, \\ v &= \frac{d\Lambda}{dx} \bar{u}, \end{aligned} \right\} \quad \text{on } y = 0. \quad (29)$$

In much the same way the conditions (12) can also be handled. To first order this gives

$$\left. \begin{aligned} u &= \mathcal{C}\tau^m, & u + \frac{\partial u}{\partial y} \Lambda &= \mathcal{C}m\tau^{m-1} \left(\frac{\partial \tau}{\partial y} \Lambda + \tau \right), \\ v &= \frac{d\Lambda}{dx} u, \end{aligned} \right\} \text{ on } y = 0. \quad (30)$$

Again, higher-order equations are easy to obtain. In what follows we shall either use Equations (29) or (30).

Next, the boundary conditions at the glacier surface are transformed to appropriate conditions at a mean surface, $y_{\text{mean}} = \hat{Y}_s^{(0)} = 1$. We begin with the kinematic condition (19). If we follow the same approach as with the boundary condition at the base, then Equation (19) must first be replaced by

$$\frac{d\hat{Y}_s}{dx} \left(\hat{u} + \frac{\partial \hat{u}}{\partial y} (\hat{Y}_s - 1) + \dots \right) \left(\hat{v} + \frac{\partial \hat{v}}{\partial y} (\hat{Y}_s - 1) + \dots \right) = a(x), \quad \text{at } y = 1. \quad (31)$$

In view of Equations (26) this can be transformed into a hierarchy of kinematic boundary conditions. Because a is of the order of 10^{-2} to 10^{-4} it is also justified to write

$$a = \epsilon A, \quad (32)$$

so that A is of order 1 at most. With the aid of Equations (32) and (26), the above boundary conditions to first order thus become

$$\left. \begin{aligned} v &= 0, & u \frac{d\hat{Y}_s}{dx} - v &= A, \end{aligned} \right\} \text{ at } y = 1. \quad (33)$$

Next, the boundary conditions of stress, Equations (14), in which all quantities carry a circumflex, must be expressed as a condition on the mean surface. To this end, notice that all quantities on the left are evaluated at the surface, while p , on the right is a given function of x . Proceeding as before we may replace the first of Equations (14) to zeroth order by

$$\left. \begin{aligned} \sigma_y &= -p, & \tau &= 0, \end{aligned} \right\} \text{ at } y = 1, \quad (34)$$

and to first order by

$$\left. \begin{aligned} \sigma_y + \frac{\partial \sigma_y}{\partial y} \hat{Y}_s - 2\tau \frac{d\hat{Y}_s}{dx} &= 0, \\ \tau + \frac{\partial \tau}{\partial y} \hat{Y}_s + (\sigma_x - \sigma_y) \frac{d\hat{Y}_s}{dx} &= 0, \end{aligned} \right\} \text{ at } y = 1. \quad (35)$$

Summarizing, Equations (27), the first of Equations (29) or (30) and of (33), and Equations (34) comprise together the zeroth-order boundary-value problem. The solution to this problem is assumed to be readily available. Knowing it, the solution to the first-order boundary-value problem can then be constructed using Equations (28), the second of Equations (29) or (30) and of (33), and (35). This boundary-value problem is *linear*.

(b) Zeroth-order solution

Consider a doubly infinite strip of thickness $\hat{Y} = 1$. For such a case it is reasonable to assume that neither stresses nor velocities depend on x and, furthermore, that $v \equiv 0$. The

fourth of Equations (27) then tells us that $\sigma_x^{(0)} = \sigma_y^{(0)}$ and the first, second, and fourth of Equations (27) can immediately be integrated to yield

$$\tau^{(0)} = \sin \gamma(1-y), \quad \sigma_x^{(0)} = \sigma_y^{(0)} = -\cos \gamma(1-y) - p, \tag{36}$$

in which the boundary conditions (34) have already been incorporated. The solution in Equation (36) is independent of the constitutive response. Notice also that τ becomes small for small γ . This should be borne in mind when higher-order solutions are constructed. To determine the velocity field notice that v was set to zero from the outset by the very assumption of the perturbation scheme (26).^{*} The u -field, on the other hand, follows from an integration of the fifth of Equations (27) which yields

$$\begin{aligned} u^{(0)} &= \bar{u} - \frac{2}{\sin \gamma} \int_{\frac{\sin \gamma}{\sin \gamma}}^{(1-y) \sin \gamma} \mathfrak{B}(\xi^2) \xi \, d\xi = \bar{u} + u_1^{(0)}(y) \\ &= \bar{u} + \frac{1}{1+k} \left\{ \frac{2 \sin^n \gamma}{n+1} [1 - (1-y)^{n+1}] + k \sin \gamma [1 - (1-y)^2] \right\}. \end{aligned} \tag{37}$$

In view of the third of Equations (27), \bar{u} must be a constant.

The solution in Equation (37) is based on a purely kinematical boundary condition. If, at the base use is made of the first of Equations (30) instead, \bar{u} in Equation (37) must be replaced by

$$\bar{u} = \mathcal{C} \sin^m \gamma. \tag{38}$$

Hence, to zeroth order, the two types of boundary conditions at the base are equivalent.

(c) *First-order solution*

To construct a solution of the first-order boundary-value problem, it is advantageous to introduce stress and stream functions. Accordingly, we set

$$\sigma_x^{(1)} = \frac{\partial^2 \phi}{\partial y^2}, \quad \sigma_y^{(1)} = \frac{\partial^2 \phi}{\partial x^2}, \quad \tau^{(1)} = -\frac{\partial^2 \phi}{\partial x \partial y}, \tag{39}$$

$$u^{(1)} = \frac{\partial \psi}{\partial y}, \quad v^{(1)} = -\frac{\partial \psi}{\partial x}, \tag{40}$$

thereby satisfying the first three of Equations (28) identically. The remaining equations then transform into the system

$$\left. \begin{aligned} \frac{\partial^2 \psi}{\partial x \partial y} &= -f(y) \left[\frac{\partial^2 \phi}{\partial x^2} - \frac{\partial^2 \phi}{\partial y^2} \right], \\ \frac{\partial^2 \psi}{\partial x^2} - \frac{\partial^2 \psi}{\partial y^2} &= g(y) \frac{\partial^2 \phi}{\partial x \partial y}, \end{aligned} \right\} \tag{41}$$

$$\left. \begin{aligned} f(y) &= \frac{1}{2} \mathfrak{B}[\sin^2 \gamma(1-y)^2], \\ g(y) &= 2 \mathfrak{B}(\sin^2 \gamma(1-y)^2) + 4 \mathfrak{B}'(\sin^2 \gamma(1-y)^2) \sin^2 \gamma(1-y)^2. \end{aligned} \right\} \tag{42}$$

⁽⁰⁾
* The assumption $v = 0$ is somewhat restricted and only justified on the basis that *a posteriori* no inconsistency in the governing equations is found. It is possible to start without such an assumption and to prove that $v = 0$.

Equations (41) are a second-order system of linear partial differential equations for ϕ and ψ with y -dependent coefficients. For a Newtonian fluid, $f(y) = \frac{1}{2}$, $g(y) = 2$, so the coefficients are constant in this case. Note that the function \mathfrak{B} may in addition carry an extra y -dependence which originates from the position dependence of the material response.

The system (41) must be complemented by appropriate boundary conditions. These are at the base: if Equations (29) are used

$$\left. \begin{aligned} \frac{\partial \psi}{\partial y} &= -2 \sin \gamma \mathfrak{B}(\sin^2 \gamma) \Lambda(x), \\ \frac{\partial \psi}{\partial y} &= -\frac{d\Lambda}{dx} \bar{u}, \end{aligned} \right\} \text{at } y = 0, \tag{43}$$

and if Equations (30) are used instead,

$$\left. \begin{aligned} \frac{\partial \psi}{\partial y} + \mathcal{C} m \sin^{m-1} \gamma \frac{\partial^2 \phi}{\partial x \partial y} \\ &= -\left[\mathcal{C} m \sin^m \gamma + \frac{2}{1+k} \{\sin^n \gamma + k \sin \gamma\} \right] \Lambda, \\ \frac{\partial \psi}{\partial x} &= -\frac{d\Lambda}{dx} \mathcal{C} \sin^m \gamma. \end{aligned} \right\} \text{at } y = 0. \tag{44}$$

At the top surface the second of Equations (33) and Equations (35) must hold. When written in terms of stream and stress functions, these conditions read

$$\left. \begin{aligned} \frac{\partial \psi}{\partial x} + [\bar{u} + u_f(1)] \frac{dY_s^{(1)}}{dx} &= A, \\ \frac{\partial^2 \phi}{\partial x^2} + \cos \gamma Y_s^{(1)} &= 0, \\ \frac{\partial^2 \phi}{\partial x \partial y} + \sin \gamma Y_s^{(1)} &= 0, \end{aligned} \right\} \text{at } y = 1, \tag{45}$$

where

$$u_f^{(0)}(1) = \frac{2}{\sin \gamma} \int_{(0)}^{\sin \gamma} \mathfrak{B}(\xi^2) \xi d\xi = \frac{1}{1+k} \left\{ \frac{2 \sin^n \gamma}{n+1} + k \sin \gamma \right\}. \tag{46}$$

Before we attempt to find a solution to the above problem we would like to emphasize that the difference in the boundary conditions at the base, Equations (43) and (44), is now explicit and, perhaps, also visible after integration. Note further, that the surface topography enters the formulation and must be determined along with the determination of the entire problem. The explicit occurrence of the accumulation rate complicates this problem slightly. It becomes easier if one sets

$$\left. \begin{aligned} Y_s^{(1)} &= Y_s^{(0)} + Y_s^a, \\ Y_s^a &= [\bar{u} + u_f]^{-1} \int_0^x A(x') dx' + A(0). \end{aligned} \right\} \tag{47}$$

This choice transforms Equation (45) into

$$\left. \begin{aligned} \frac{\partial \psi}{\partial x} + [\bar{u} + u_t] \frac{d^{(1)} Y_s^0}{dx} &= 0, \\ \frac{\partial^2 \phi}{\partial x^2} + \cos \gamma Y_s^{(1)} &= -\cos \gamma Y_s^a, \\ \frac{\partial^2 \phi}{\partial x \partial y} + \sin \gamma Y_s^{(1)} &= -\sin \gamma Y_s^a, \end{aligned} \right\} \text{ at } y = 1, \tag{48}$$

in which Y_s^a is a known function of x .

Before we proceed, we indicate how the general steady-state problem with steady-state accumulation rate can be solved. To this end, it is not hard to see that the influence of the bedrock undulations and that of a steady accumulation rate can be fully separated. Indeed, let $\phi = \phi_\Lambda + \phi_A$, $\psi = \psi_\Lambda + \psi_A$, $Y_s^0 = Y_\Lambda^0 + Y_A^0$, where the functions with index Λ are solutions to Equations (41), (43) or (44) and (48) if $A = 0$, whereas those with index A are the corresponding solutions for $A \neq 0$ but $\Lambda = 0$. Adding these solutions then gives the general solution for the general steady-state problem. Thus question (ii) posed in the introduction can be answered in an affirmative form.

(d) *Approximate integration scheme*

The differential equations (41) are not easy to integrate, in general; it is thus appropriate to reserve a separate section for this integration. Here we would like to demonstrate, how the first-order equations can be integrated approximately. Such an approximate scheme was introduced by Budd (1970[a]) without any mention of its approximate character. Because of the prominence of his article and our conclusions in this article as to the falsity of the approximate integration scheme, it will be briefly outlined here. To this end we write Equations (28) in terms of deviator stresses $\sigma_x', \sigma_y', \tau$ and pressure p , rather than stresses (see, e.g. Equations (7) and (8)). *Postulating that the perturbation pressure vanishes* $p = 0$, the first-order momentum equations can be satisfied identically, provided that

$$\sigma_x' = \frac{\partial^2 \phi'}{\partial y^2}, \quad \sigma_y' = \frac{\partial^2 \phi'}{\partial x^2}, \quad \tau = -\frac{\partial^2 \phi'}{\partial x \partial y}. \tag{49}$$

From the continuity equation and the constitutive relations involving σ_x' and σ_y' it then follows also that

$$\sigma_x' + \sigma_y' = 0, \quad \text{or } \Delta \phi' = 0, \tag{50}$$

whereas the constitutive relation for τ implies that

$$\frac{\partial^2 \psi'}{\partial x^2} - \frac{\partial^2 \psi'}{\partial y^2} = g(y) \frac{\partial^2 \phi'}{\partial x \partial y} \tag{51}$$

where ψ' is the stream function of this approximate problem (primes will indicate that the perturbation pressures p have been set to zero). The boundary conditions are the same as before, and all we need to do is replace ϕ, ψ and Y_s by ϕ', ψ' and Y_s' , respectively.

The differential equations which must be solved in this approximate approach are Equations (50) and (51). As compared to Equations (41) they carry the advantage of being

separated insofar as the general solution of the Laplace equation for the stress function can be constructed in advance and may, in a second step, then be used as a forcing function in the wave equation (51) for the stream function ψ' . Of course, the suitability of the approximate boundary-value problem in which $p = 0$, must be tested. It is not correct, in general, as we have clearly outlined above. Budd's approach is less systematic also in other points. For instance, his treatment of the boundary conditions is quite different.

3.2. Harmonic perturbations from uniform flow for zero accumulation rate

In this section we shall solve the boundary-value problem for the special case that $A(x) = 0$. We assume that $\Lambda(x)$ is given as a Fourier series,

$$\Lambda(x) = \sum_{\nu=1}^{\infty} B_{\nu} \cos(\omega_{\nu} x). \quad (52)$$

It suffices only to look at a single frequency, and so we will simply write

$$\Lambda(x) = \cos \omega x. \quad (53)$$

We also expect the surface topography to be sinusoidal, but shifted with respect to Equation (53), so that

$$Y_s^{(1)} = h_1 \cos \omega x + h_2 \sin \omega x. \quad (54)$$

The functions

$$\left. \begin{aligned} \mathcal{F}(\omega) &= (h_1^2(\omega) + h_2^2(\omega))^{\frac{1}{2}}, \\ \varphi(\omega) &= \tan^{-1}(h_2(\omega)/h_1(\omega)), \end{aligned} \right\} \quad (55)$$

will be called the *filter function* and the *phase shift*, respectively. The former determines that fraction of the protuberance amplitude that is, at a certain frequency, carried over to the surface. It could also be interpreted as the ratio of the surface to the basal amplitude of the *inclination*. According to the observations and the calculations of Budd (1970[a]) this function should have a maximum for wavelengths which are between 3 to 4 times the glacier thickness. The phase shift, on the other hand, is close to $\pi/2$.

Because the Equations (50) and (51) will be shown to lead to false results and the approximate solution will only be used to demonstrate this point, the derivation of the solution will not be presented here. Interested readers may consult the first author (Hutter). Here we only list the relationships for h_1 and h_2 which will allow the evaluation of the filter and the phase-shift functions when Glen's flow law is used. One obtains

$$h_1 = \{(\sin \gamma) V_p(1) + [\bar{u} + u_f]^{(0)}\} \left\{ \bar{u} \cos \omega + \frac{2(\sin \gamma) \mathfrak{B}(\sin^2 \gamma) \sin \omega}{\omega} \right\} / \Delta, \quad (56)$$

$$h_2 = -(\cos \gamma) V_m(1) \left[\bar{u} \cos \omega + \frac{2(\sin \gamma) \mathfrak{B}(\sin^2 \gamma) \sin \omega}{\omega} \right] / \Delta, \quad (57)$$

in which

$$\Delta = [(\sin \gamma) V_p(1) + \bar{u} + u_f]^{(0)2} + (\cos^2 \gamma) V_m(1). \quad (58)$$

$V_p(y)$ and $V_m(y)$ are the functions

$$V_p(y) \equiv \frac{n \sin^{n-1} \gamma}{\omega^{n+2}} [I_{n-1}(\omega, \omega(1-y)) - I_{n-1}(\omega(1-y), \omega(1-y))], \quad (59)$$

$$V_m(y) \equiv \frac{n \sin^{n-1} \gamma}{\omega^{n+2}} [\mathcal{J}_{n-1}(\omega, \omega(1-y)) - \mathcal{J}(\omega(1-y), \omega(1-y))], \quad (60)$$

where

$$\left. \begin{aligned} I_0(\xi, x) &= \sin(\xi-x) \sinh \xi - \cos(\xi-x) \cosh \xi, \\ \mathcal{J}_0(\xi, x) &= -\sin(\xi-x) \cosh \xi + \cos(\xi-x) \sinh \xi, \end{aligned} \right\} \quad (61)$$

$$\left. \begin{aligned} I_1(\xi, x) &= \xi \sin(\xi-x) \cosh \xi - \cos(\xi-x) \sinh \xi(\xi+1), \\ \mathcal{J}_1(\xi, x) &= -\xi \sin(\xi-x) \sinh \xi + \cos(\xi-x) \cosh \xi(\xi-1), \end{aligned} \right\} \quad (62)$$

$$\left. \begin{aligned} I_2(\xi, x) &= 2\xi \cos(\xi-x) \sinh \xi + \sin(\xi-x) \sinh \xi(\xi^2-1) - \\ &\quad - \cos(\xi-x) \cosh \xi(\xi^2+1), \\ \mathcal{J}_2(\xi, x) &= -2\xi \cos(\xi-x) \cosh \xi - \sin(\xi-x) \cosh \xi(\xi^2-1) + \\ &\quad + \cos(\xi-x) \sinh \xi(\xi^2+1). \end{aligned} \right\} \quad (63)$$

With these formulas the evaluation of the filter function \mathcal{F} becomes a routine matter. We shall postpone the presentation of numerical results until the next chapter.

(b) *Exact solution for a Navier–Stokes fluid*

Unfortunately, an exact integration of Equations (41) is hardly possible for general non-linear material behaviour. This case, although being of a somewhat academic nature, is nevertheless important because it allows a check of the numerical scheme needed for the non-linear fluid. Since for this case $f(y) = \frac{1}{2}$ and $g(y) = 2$ the coefficients in Equation (41) are constant. With

$$\phi = \Phi_1(y) \cos \omega x + \Phi_2(y) \sin \omega x, \quad \psi = \Psi_1(y) \cos \omega x + \Psi_2(y) \sin \omega x, \quad (64)$$

the emerging differential equations for the coefficient functions assume the form

$$\left. \begin{aligned} -2\omega \frac{d\Psi_1}{dx} &= \omega^2 \Phi_2 + \frac{d^2 \Phi_2}{dy^2}, & 2\omega \frac{d\Phi_1}{dx} &= \omega^2 \Psi_2 + \frac{d^2 \Psi_2}{dy^2}, \\ 2\omega \frac{d\Psi_2}{dx} &= \omega^2 \Phi_1 + \frac{d^2 \Phi_1}{dy^2}, & -2\omega \frac{d\Phi_2}{dx} &= \omega^2 \Psi_1 + \frac{d^2 \Psi_1}{dy^2}. \end{aligned} \right\} \quad (65)$$

The boundary conditions (43) and the second and third of (45) on the other hand become: at $y = 0$,

$$\left. \begin{aligned} \frac{d\Psi_1}{dy} &= -2 \sin \gamma B, & \frac{d\Psi_2}{dy} &= 0, \\ \Psi_1 &= -\bar{u}B, & \Psi_2 &= 0, \end{aligned} \right\} \quad (66)$$

at $y = 1$,

$$\left. \begin{aligned} \Phi_1 &= \frac{h_1 \cos \gamma}{\omega^2}, & \Phi_2 &= \frac{h_2 \cos \gamma}{\omega^2}, \\ \frac{d\Phi_1}{dy} &= \frac{h_2 \sin \gamma}{\omega}, & \frac{d\Phi_2}{dy} &= -\frac{h_1 \sin \gamma}{\omega}. \end{aligned} \right\} \quad (67)$$

The form of Equations (65) suggests searching for exponential solutions. Indeed, the reader may check himself that

$$\left. \begin{aligned} \Psi_1 &= (a_0 + a_1 y) \exp(\omega y) + (\bar{a}_0 + \bar{a}_1 y) \exp(-\omega y), \\ \Psi_2 &= (b_0 + b_1 y) \exp(\omega y) + (\bar{b}_0 + \bar{b}_1 y) \exp(-\omega y), \\ \Phi_1 &= (b_0 + b_1 y) \exp(\omega y) - (\bar{b}_0 + \bar{b}_1 y) \exp(-\omega y), \\ \Phi_2 &= -(a_0 + a_1 y) \exp(\omega y) + (\bar{a}_0 + \bar{a}_1 y) \exp(-\omega y), \end{aligned} \right\} \quad (68)$$

is the most general solution of Equations (65). The eight unknown coefficients are determinable from the eight boundary conditions (66) and (67). The corresponding calculations are very tedious, even though they are straightforward, so that it does not seem to be appropriate to present them explicitly. After lengthy manipulations one obtains:

$$\left. \begin{aligned} a_0 &= -h_1\Omega_1 \sin \gamma - h_2\Omega_2 \cos \gamma - B\Omega_3 \sin \gamma - \frac{1}{2}B\bar{u}(\Omega_4 + 1), \\ \bar{a}_0 &= h_1\Omega_1 \sin \gamma + h_2\Omega_2 \cos \gamma + B\Omega_3 \sin \gamma + \frac{1}{2}B\bar{u}(\Omega_4 - 1), \\ a_1 &= h_1\Omega_5^- \sin \gamma + h_2\Omega_6^- \cos \gamma + 2B\Omega_7^- \sin \gamma + B\bar{u}\omega\Omega_8^-, \\ \bar{a}_1 &= -h_1\Omega_6^+ \sin \gamma + h_2\Omega_5^+ \cos \gamma - 2B \sin \gamma \Omega_8^+ + B\bar{u}\Omega_7^+ \omega, \end{aligned} \right\} \quad (69)$$

and

$$\left. \begin{aligned} b_0 &= -b_0 = -h_1\Omega_2 \cos \gamma + h_2\Omega_1 \sin \gamma, \\ b_1 &= -h_1\Omega_6^- \cos \gamma + h_2\Omega_5^- \sin \gamma, \\ \bar{b}_1 &= -h_1\Omega_5^+ \cos \gamma - h_2\Omega_6^+ \sin \gamma, \end{aligned} \right\} \quad (70)$$

where

$$\left. \begin{aligned} \Omega_1 &= \cosh \omega / (2\omega\bar{\Delta}), \\ \Omega_2 &= (\cosh \omega + \omega \sinh \omega) / (2\omega^2\bar{\Delta}), \\ \Omega_3 &= \omega / \bar{\Delta}, \\ \Omega_4 &= (\omega - \cosh \omega \sinh \omega) / \bar{\Delta}, \\ \Omega_5^\pm &= (\cosh \omega + \omega \exp(\pm\omega)) / (2\omega\bar{\Delta}), \\ \Omega_6^\pm &= (\cosh \omega - \omega \exp(\pm\omega)) / (2\omega\bar{\Delta}), \\ \Omega_7^\pm &= (\omega - \exp(\pm\omega) \cosh \omega) / (2\bar{\Delta}), \\ \Omega_8^\pm &= (\omega + \exp(\pm\omega) \cosh \omega) / (2\bar{\Delta}), \end{aligned} \right\} \quad (71)$$

and

$$\bar{\Delta}(\omega) = \cosh^2 \omega + \omega^2. \quad (72)$$

With the stress and stream functions being determined as functions of the surface and bottom amplitudes h_1 , h_2 and $B = 1$, the former two follow from Equations (48) by mere substitution. If this is done, we obtain

$$h_1 = \frac{4\omega\Omega_1 \sin \gamma + 2\bar{u}\omega^2\Omega_2}{(\sin \gamma + \bar{u})^2 + (\cos^2 \gamma) \Omega_4^2/\omega^4} (\sin \gamma + \bar{u}), \quad (73)$$

$$h_2 = \frac{4\omega\Omega_1 \sin \gamma + 2\bar{u}\omega^2\Omega_2}{(\sin \gamma + \bar{u})^2 + (\cos^2 \gamma) \Omega_4^2/\omega^4} \cos \gamma \frac{\Omega_4}{\omega^2}. \quad (74)$$

The filter function \mathcal{F} and the phase lag angle φ are therefore given by

$$\left. \begin{aligned} \mathcal{F} &= \frac{|4\omega\Omega_1 \sin \gamma + 2\bar{u}\omega^2\Omega_2|}{[(\sin \gamma + \bar{u})^2 + (\cos^2 \gamma) \Omega_4^2/\omega^4]^{\frac{1}{2}}}, \\ \tan \varphi &= \frac{\Omega_4 \cos \gamma}{\omega^2(\sin \gamma + \bar{u})^2}. \end{aligned} \right\} \quad (75)$$

This completes the construction of the exact solution.

(c) Numerical solution for non-linear rheology

For the harmonic bed undulations (Equation (53)) the stream and stress functions may be assumed to have the form of Equations (64). When substituted into Equations (41), the following system of ordinary differential equations for the unknown functions Φ_1 , Φ_2 , Ψ_1 and Ψ_2 are obtained:

$$\left. \begin{aligned}
 g_1 &\equiv -\omega^2 f_1 - \frac{\cos \gamma}{(\bar{u} + u_t)} f_3 = 0, \\
 g_2 &\equiv -\omega^2 f_2 - \frac{\cos \gamma}{(\bar{u} + u_t)} f_4 = 0, \\
 g_3 &\equiv -\omega f_5 - \frac{\sin \gamma}{(\bar{u} + u_t)} f_4 = 0, \\
 g_4 &\equiv \omega f_6 - \frac{\sin \gamma}{(\bar{u} + u_t)} f_3 = 0.
 \end{aligned} \right\} \quad (82)$$

To solve the boundary-value problem (80), (81), and (82), it is best to integrate it numerically with, say, a Runge-Kutta scheme. Because of the linearity of the problem the principle of superposition can be used. Accordingly, we solve Equation (80) with the following five different initial conditions:

$$\left. \begin{aligned}
 f_0^0 &= (0, 0, -u, 0, 0, 0, -2 \sin \gamma \mathfrak{B}(\sin^2 \gamma), 0), \\
 f_0^1 &= (1, 0, 0, 0, 0, 0, 0, 0), \\
 f_0^2 &= (0, 1, 0, 0, 0, 0, 0, 0), \\
 f_0^3 &= (0, 0, 0, 0, 1, 0, 0, 0), \\
 f_0^4 &= (0, 0, 0, 0, 0, 1, 0, 0).
 \end{aligned} \right\} \quad (83)$$

For each of these initial conditions the values of g_1, \dots, g_4 in Equations (82) can be evaluated; they are

$$g_1^0, g_1^1, g_1^2, g_1^3, g_1^4, g_2^0, \dots, g_2^4, \dots$$

The correct solution is obtained if

$$g_i^0 + g_i^j X_j = 0, \quad i, j = (1, 2, 3, 4),$$

which is a linear system of four equations for the unknowns X_j . Once they are determined we may integrate Equation (80) once more with the initial condition

$$f_0 = (X_1, X_2, -\bar{u}, 0, X_3, X_4, -2 \sin \gamma \mathfrak{B}(\sin^2 \gamma), 0),$$

to obtain the final solution for the stress and stream functions. First-order stresses and velocities are then obtained by simple differentiations. A program was written solving the boundary-value problem in the indicated manner. Solutions of it can be obtained for various different physical situations, some of which we shall discuss below.

3.3. Effect of steady accumulation rate

In the last subsection accumulation was set to zero, but bottom undulations were present. Here we shall assume a non-vanishing accumulation rate, but shall set all bottom undulations aside. If both are small their effects can be superimposed, as we have already seen above.

It was mentioned earlier that the zeroth-order solution constructed in the Equations (36) and (37) did not account for geometric effects of steady-state accumulation rate. This limits considerations to length scales which are not asymptotically large as compared to glacier

thicknesses. The effect of accumulation rate on length scales somewhat larger than glacier thickness may, however, still be analysed.

To solve the boundary-value problem described by Equations (41), (43), and (48) we shall set $\Lambda \equiv 0$ and, further assume that $Y_s^{(1)}$ be given in terms of a Fourier cosine series. It then suffices to restrict considerations to the single component

$$Y_s^{(1)} = H \cos \omega x.$$

Stress and stream functions may then be given as shown in (64) and $Y_s^{(1)}$ as shown in Equations (54). Substituting these into Equations (41), (43), and (48) it is then easily seen that the following boundary-value problem must be solved:

$$\left. \begin{aligned} \frac{d\mathbf{f}}{dy} &= \mathbf{A}(y) \mathbf{f}, \\ f_3 = f_4 = f_7 = f_8 &= 0, \quad \text{at } y = 0, \\ g_1 = -\cos \gamma H, \quad g_2 = g_3 &= 0, \quad g_4 = -\sin \gamma H, \quad \text{at } y = 1. \end{aligned} \right\} \quad (84)$$

In these equations \mathbf{f} is defined in Equation (79), $\mathbf{A}(y)$ in (80 b) and g_1, \dots, g_4 in (82). The construction of numerical solutions to the above boundary-value problem follows essentially the approach of Section 3.2(c) and will not be repeated here.

For a *Navier-Stokes fluid*, an analytical solution to the boundary value problem (84) can be constructed, which, in approach is very similar to that in Section 3.2(b). These solutions were used as a further check of the numerical integration scheme. For brevity they will not be presented here.

3.4. Influence of differences in boundary conditions at the base

All the foregoing calculations are based on the kinematic boundary conditions (29). We have seen that, to zeroth order, this condition is equivalent to (30); but when first-order equations are considered, (29) and (30) differ from each other. It is interesting to investigate in what respect the difference in boundary conditions results in differences in the state of first-order stresses and velocities.

The boundary-value problem that must be solved is now Equations (41), (44), and (48). For vanishing accumulation rate and harmonic solutions (54) and (64) the system (80) evolves as do the boundary conditions (82), but Equations (81) must be replaced by

$$\left. \begin{aligned} f_3 = -\bar{u}, \quad f_7 + \mathfrak{X} f_6 &= -m\bar{u} - 2 \sin \gamma \mathfrak{B}(\sin^2 \gamma), \\ f_4 = 0, \quad f_8 - \mathfrak{X} f_5 &= 0, \end{aligned} \right\} \quad \text{at } y = 0, \quad (85)$$

where $\mathfrak{X} := m\bar{u}\omega/\sin \gamma$ and $\bar{u} = \mathcal{C} \sin^m \gamma$. The form of the third and fourth of Equations (85) suggests the introduction of a new vector \mathcal{F} by

$$\mathcal{F} = (\mathcal{F}_1, \mathcal{F}_2, \dots, \mathcal{F}_8)^T = (f_1, f_2, \dots, f_6, f_7 + \mathfrak{X} f_6, f_8 - \mathfrak{X} f_5)^T. \quad (86)$$

It is then straight forward to show that the differential equations (80) may be written as

$$\frac{d\mathcal{F}}{dy} = \mathfrak{U}(y) \mathcal{F}, \quad (87)$$

with

$$\mathfrak{U} = \left[\begin{array}{cccc|cccc} & & & & 1 & 0 & 0 & 0 \\ & & & & 0 & 1 & 0 & 0 \\ & & & & 0 & -\kappa & 1 & 0 \\ & & & & \kappa & 0 & 0 & 1 \\ \hline -\omega^2 & 0 & 0 & 0 & \frac{\omega\kappa}{f(y)} & 0 & 0 & \frac{\omega}{f(y)} \\ 0 & -\omega^2 & 0 & 0 & 0 & \frac{\omega\kappa}{f(y)} & -\frac{\omega}{f(y)} & 0 \\ 0 & -\omega^2\kappa & -\omega^2 & 0 & 0 & \omega\left(\frac{\kappa^2}{f(y)} - g(y)\right) & -\frac{\omega}{f(y)} & 0 \\ \omega^2\kappa & 0 & 0 & -\omega^2 & \omega\left(g(y) - \frac{\kappa^2}{f(y)}\right) & 0 & 0 & -\frac{\omega}{f(y)} \end{array} \right] \quad (88)$$

This system must be subject to the boundary conditions

$$\mathcal{F}_3 = -\bar{u}, \quad \mathcal{F}_4 = \mathcal{F}_7 = \mathcal{F}_8 = 0, \quad \text{at } y = 0, \quad (89)$$

and

$$\left. \begin{aligned} \mathcal{G}_1 &:= -\omega^2 \mathcal{F}_1 - \frac{\cos \gamma}{(0)} \mathcal{F}_3 = 0, \\ \mathcal{G}_2 &:= -\omega^2 \mathcal{F}_2 - \frac{\cos \gamma}{(0)} \mathcal{F}_4 = 0, \\ \mathcal{G}_3 &:= -\omega \mathcal{F}_5 - \frac{\sin \gamma}{(0)} \mathcal{F}_4 = 0, \\ \mathcal{G}_4 &:= +\omega \mathcal{F}_6 - \frac{\sin \gamma}{(0)} \mathcal{F}_3 = 0, \end{aligned} \right\} \text{at } y = 1. \quad (90)$$

The boundary-value problem (87) to (90) agrees formally with that of Section 3.2(c). The technique for numerical solution therefore also agrees with the one presented there.

4. RESULTS

Several interesting features can be obtained from a numerical exploitation of the theoretical results derived above. These will be discussed now in due order. We begin with the effect of bottom undulations, continue to discuss the influence of accumulation rate and the effects of sliding at the bottom, and end by discussing the effect of temperature variations on the longitudinal strain effects in cold ice. In the following, accumulation is neglected except where explicitly stated.

4.1. *Transfer of bottom protuberances to the surface*

We begin with the presentation of the filter function \mathcal{F} and the phase lag angle φ . Analytical expressions were obtained for a Newtonian fluid and for the case that no simplifications were introduced in the stress representations. It was shown, moreover, that analytical solutions for the first-order stresses and velocities could also be obtained when first-order pressure corrections were discarded. Figures 2 and 3 show the corresponding results. In Figure 2a the filter function \mathcal{F} is plotted against a dimensionless wavelength $\lambda = 2\pi/\omega$ for the case when there is no slip along the basal surface. It is clearly seen that the transfer of bottom undulations depends strongly on the mean inclination γ of the ice slope. Generally, \mathcal{F} grows with growing λ , and from the graph it appears that it does so in a monotonic way. Consequently, small-wavelength protuberances are filtered out, but large wavelengths are transferred to the surface, even though in an attenuated form. This result is surprising as it is

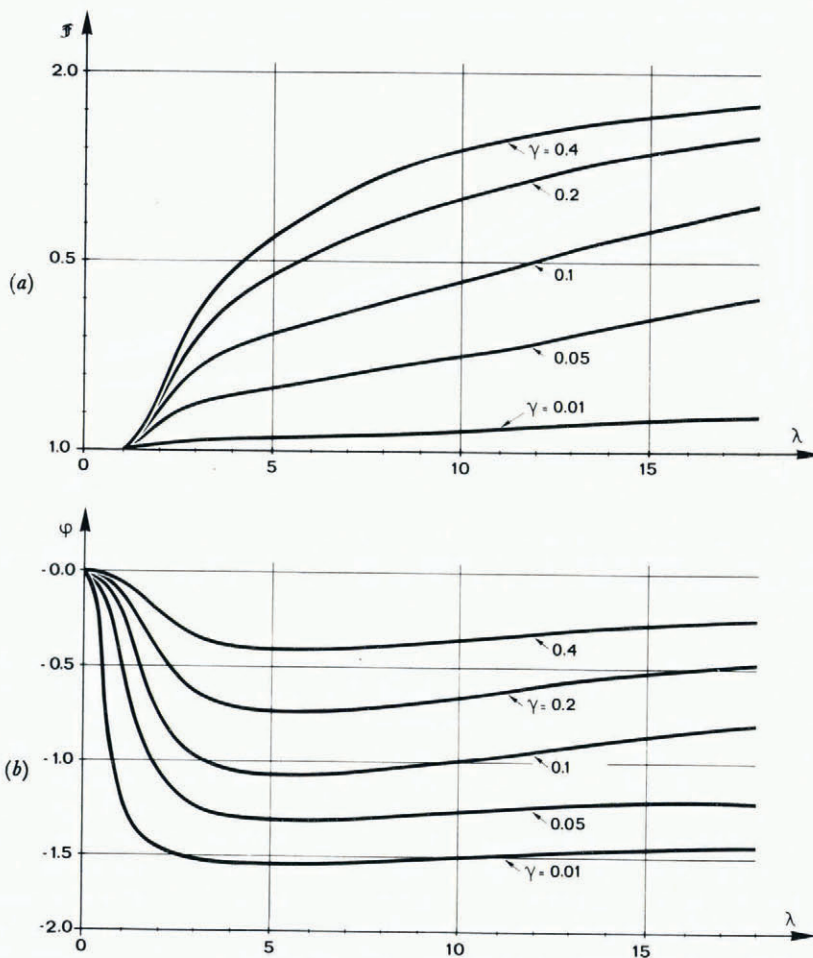


Fig. 2. (a) Filter function \mathcal{F} plotted against dimensionless wavelength $\lambda = 2\pi/\omega$ for a Navier-Stokes liquid and parameterized for various mean inclinations γ . No slip at the bottom. (b) Phase lag angle φ for the same situation as in (a).

entirely different from Budd's (1970[a]) prediction. His results look qualitatively like those in Figure 3 in which the filter function \mathcal{F} , as obtained from the approximate solution of Section 3.2(a) shows a clear maximum at a preferred wavelength λ . The similarity of this curve with that of Budd stems from the fact that both Budd and we neglect the perturbation pressure in the calculations leading to Figure 3. Quantitative differences, on the other hand, emanate from differences in the boundary conditions at the free surface. Incidentally, the behaviour of \mathcal{F} at wavelengths which are below $\lambda = 0.7$ is somewhat awkward, because \mathcal{F} does not monotonically decrease with decreasing λ but shows an intermediate maximum at $\lambda \approx 0.45$. This indicates that the approximate solution must fail at small wavelengths. Since the least approximations have been made in the calculations leading to Figure 2 we must reject both Budd's and our approximate solutions. We have also shown in Figure 2b the phase lag angle φ . Except for small wavelengths it is nearly independent of wavelength and depends on the inclination γ . For very small γ this phase lag is close to $-\pi/2$.

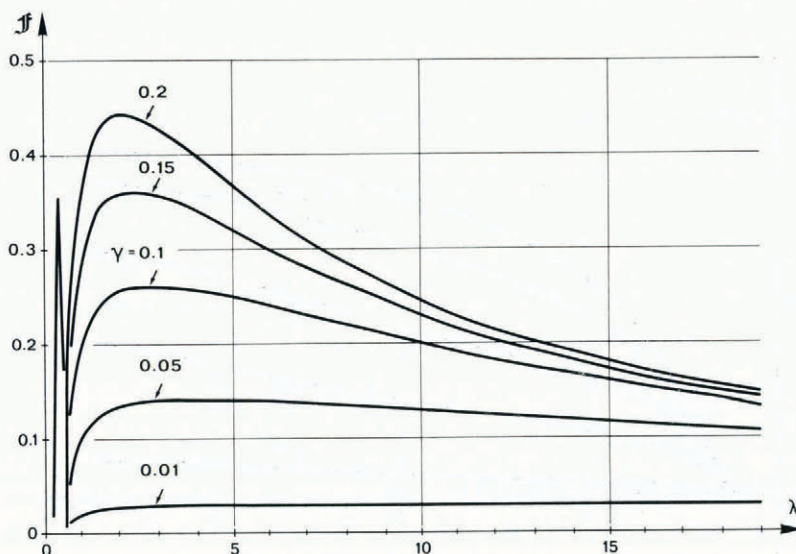


Fig. 3. Filter function \mathcal{F} plotted against dimensionless wavelength $\lambda = 2\pi/\omega$ for a Navier-Stokes liquid and parameterized for various mean inclinations γ . Approximate solution as derived in Section 3.2a. No slip at the bottom. The spike at small values of λ indicates a failure of the approximate integration procedure at small wavelengths.

Results have also been calculated for an ice slope sliding down its bed. As far as \mathcal{F} and φ are concerned these are virtually indistinguishable from those obtained with the no-slip condition, hence no further results need be presented.

There is still the possibility of a dominant transfer of bottom undulations to the surface at a certain distinct wavelength when non-linear constitutive behaviour is considered. That this possibility can also be excluded is demonstrated in Figure 4. In Figures 4a and 4b the filter function \mathcal{F} is displayed as a function of $\lambda = 2\pi/\omega$ as obtained by use of the numerical integration scheme described above for a generalized Glen flow law with exponent $n = 2$ and 3 (the value of \mathbf{k} is 10^{-2} for $n = 2$ and $\mathbf{k} = 10^{-3}$ for $n = 3$). As can be seen from a comparison of Figures 2a and 4a and b, \mathcal{F} grows monotonically with increasing λ . Moreover, transfer of bottom undulations to the surface is enhanced with increasing n . Hence here too, predominance of certain wavelengths at the top surface cannot be correlated with bedrock undulations. Their cause must be searched for elsewhere.

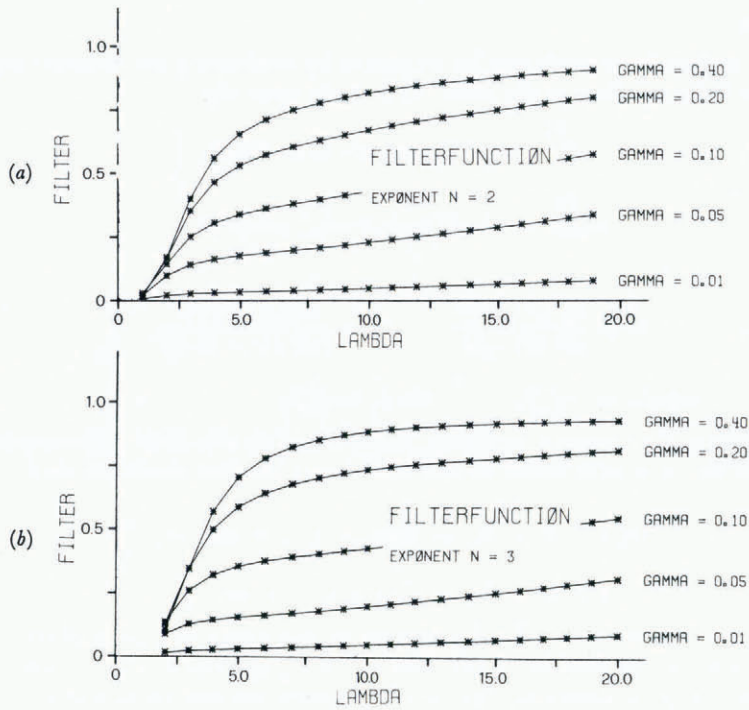


Fig. 4. Filter function \mathcal{F} plotted against $\lambda = 2\pi/\omega$ for various mean inclinations γ and for a generalized Glen flow law with $n = 2$, (a), and $n = 3$, (b). The value of k was $(10^{-2}, 10^{-3})$ for $n = (2, 3)$. At small wavelengths (not plotted) the value of \mathcal{F} is nearly zero. No slip at the bottom.

Phase lag angles could also be shown for the above cases, yet differences from Figure 2b are insignificant so corresponding graphs are not shown.

Finally, the influence of the k term in the generalized Glen flow law may be estimated from Figure 5, which shows the filter function for $n = 3$ and $\gamma = 0.1$ for various values of k . It is seen that \mathcal{F} depends significantly on k provided that $k > 10^{-4}$. This points to the importance of the quasi-Newtonian behaviour of the material response introduced in Section 2.1 and does not point to a possible failure of the numerical scheme, which is stable for most wavelengths even for very small values of k .

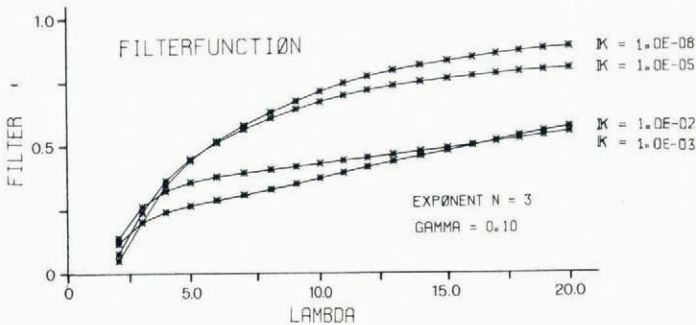


Fig. 5. Filter function \mathcal{F} plotted for $\gamma = 0.1$ against $\lambda = 2\pi/\omega$ parameterized for various values of k .

4.2. Basal stresses

The effect of longitudinal strain can be estimated by evaluating the stresses tangential and perpendicular to the boundary at the base. It is easily seen that

$$\left. \begin{aligned} \tau/\tau &= 1 + \epsilon \Theta \cos(\omega x - \phi_\tau) + O(\epsilon^2), \\ \sigma_y/\sigma_y &= 1 + \epsilon \Sigma \cos(\omega x - \phi_\sigma) + O(\epsilon^2), \end{aligned} \right\} \quad (91)$$

where

$$\left. \begin{aligned} \Theta &\equiv \frac{\omega(f_5^2 + f_6^2)^{\frac{1}{2}}}{\sin \gamma(1-y)}, & \Sigma &\equiv \frac{\omega^2(f_1^2 + f_2^2)^{\frac{1}{2}}}{\cos \gamma(1-y) + p}, \\ \tan \phi_\tau &= -f_5/f_6, & \tan \phi_\sigma &= f_2/f_1. \end{aligned} \right\} \quad (92)$$

Θ and Σ depend on y , $\lambda = 2\pi/\omega$, p , γ , and \bar{u} (which through Equation (38) may be expressed in terms of \mathcal{C} and m). p is very small; at the base $y = 0$ we may therefore set $p = 0$. Consequently,

$$\left. \begin{aligned} \Theta(y=0) &= \Theta_0 = \Theta_0(\lambda, \gamma, \bar{u} = \mathcal{C} \sin^m \gamma), \\ \Sigma(y=0) &= \Sigma_0 = \Sigma_0(\lambda, \gamma, \bar{u} = \mathcal{C} \sin^m \gamma). \end{aligned} \right\} \quad (93)$$

These functions are indicators for the significance of the effect of longitudinal strains upon basal shear and for the limitations of the mathematical approach applied herein. Within errors $O(\epsilon^2)$, the second of Equations (91) gives the pressure normal to the base. Thus, we must necessarily have $(\Theta_0, \Sigma_0) = O(1)$. The functions Θ_0 and ϕ_τ are plotted for no slip and for various inclination angles γ , in Figures 6 and 7 against λ . It is clearly seen that perturbation amplitudes for the shear stresses grow with decreasing wavelength. They are generally larger the smaller γ . Of course, this property is directly traceable to the first of Equations (92). The singularity associated with the limit $\gamma \rightarrow 0$ is, however, spurious as explained previously already. At large λ , Θ_0 grows with decreasing γ , but at small wavelengths, where the shear stresses become large and a singularity seems to develop, it is the reverse. The singularity is not a numerical peculiarity, because it has also been obtained for $n = 1$ using the analytical solution represented by Equations (64) to (74). It follows, therefore, that the perturbation solution must break down for wavelengths $\lambda < c. 1$. The reader should be aware of this limitation of our perturbation scheme; that it develops is not surprising, however. For if at

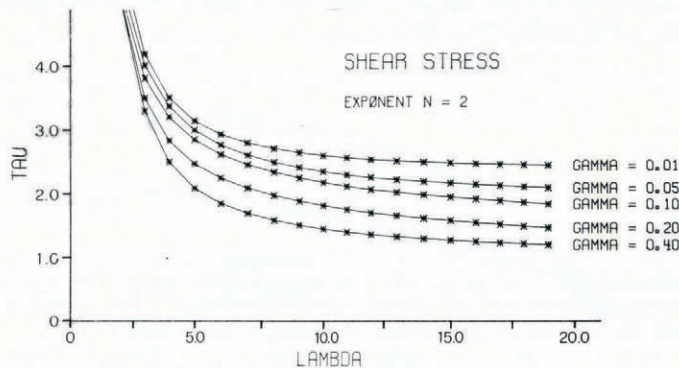


Fig. 6. First order amplitude of shear stress Θ_0 plotted against λ and for various values of the inclination angle γ . The exponent in the generalized Glen flow law is $n = 2$, and $k = 10^{-2}$. No slip at the bottom.

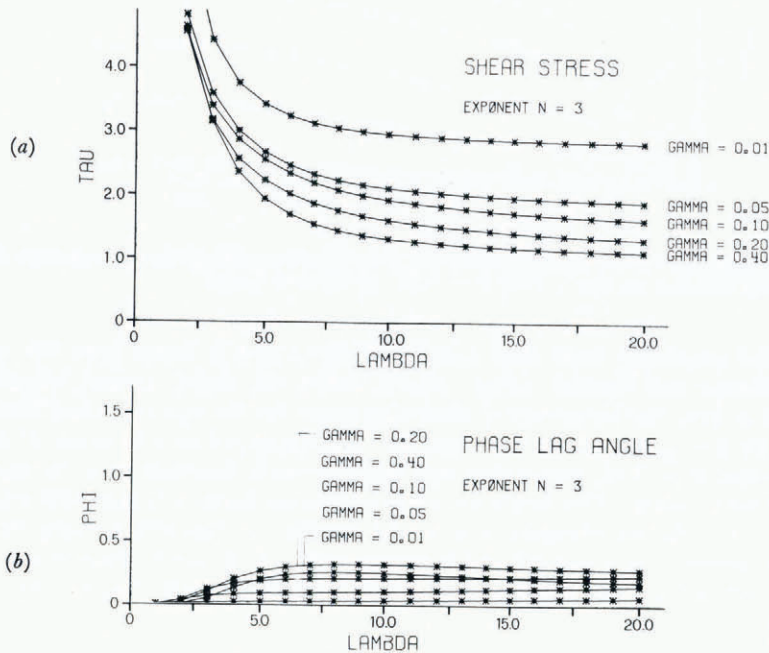


Fig. 7. (a) Same as Figure 6, but for $n = 3$ and $k = 10^{-3}$. (b) Phase lag angle ϕ_r for the basal shear stresses plotted against λ and for various values of γ . The exponent in the generalized Glen flow law is $n = 3$ and $k = 10^{-3}$. No slip at the bottom.

fixed amplitude undulation wavelengths become smaller and smaller, their higher derivatives will correspondingly increase and eventually no longer satisfy the assumptions of small perturbation theory. Of course, the validity also depends on the value of ϵ .

For $\bar{u} = 0$ phase lag angles ϕ_r are shown in Figure 7b. They are all nearly independent of λ and close to zero. Similar results have also been obtained for $n = (1, 2)$ for which reason the corresponding graphs will be omitted.

Besides shear, the other significant stress component is the stress normal to the surface. To order ϵ^2 this is given by σ_y . For no slip and $n = (2, 3)$ its first-order amplitudes Σ_0 (see Equations (93)) are plotted in Figures 8 and 9a. Generally, a weak dependence of Σ_0 on λ can be observed, and, except for $\gamma > c. 0.2$ and $\lambda < 3$, Σ_0 increases with increasing λ . Only for $\gamma > c. 0.2$ and $\lambda < 3$ is a growth of Σ_0 with decreasing λ observed. This seems to be a property of the non-linear material behaviour, because it did not occur for $n = 1$. Particularly interesting in Figures 8 and 9a is the fact that for all wavelengths $\lambda > 2$, and for all inclination angles $\Sigma < 1$. Only at small wavelengths can the onset of a singularity be observed. Hence,

the sign of σ_y will, for $\epsilon < 1$ and for most wavelengths, be that of $\sigma_y^{(0)}$. Failure of the perturbation scheme used above therefore does not occur because of a possible locally induced tension, but rather by the first-order shear stresses, which become infinitely large at smaller λ s. It is interesting that in cold ice sheets, in which temperature variations are taken into account by appropriate variations of the Arrhenius factor, basal stresses are considerably reduced so that

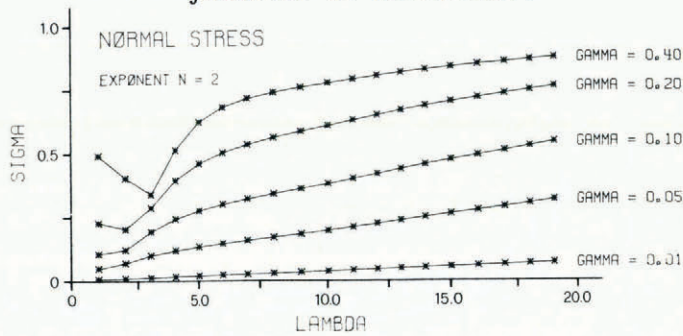


Fig. 8. First-order amplitude of normal stress Σ_0 plotted against λ and for various values of the inclination angle γ . The exponent in the generalized Glen flow law is $n = 2$ and $k = 10^{-2}$. No slip at the bottom.

the perturbation approach used here and valid for $\lambda > c.2$ holds even for wavelengths $\lambda \approx 0.5$, see Hutter and Spring (1979). Quite unlike the shear stresses, whose phase lag angle was close to zero, the behaviour of phase lag angle for the σ_y stresses is different. Figure 9b, in which, for $n = 3$, the phase lag angle for σ_y is displayed as a function of λ and parameterized for various γ . This shows that this angle strongly depends on γ ; in particular, for small γ , it is close to $-\pi/2$. For larger inclinations and for large λ it increases and is close to zero for $\gamma > c.0.4$. At wavelengths $\lambda \approx 5$ and smaller, it depends strongly on λ and all curves seem to converge to $-\pi/2$ when λ becomes unity. This same characteristic behaviour has also been observed for $n = 1$ and $n = 2$, which is the reason why results are not presented for these cases.

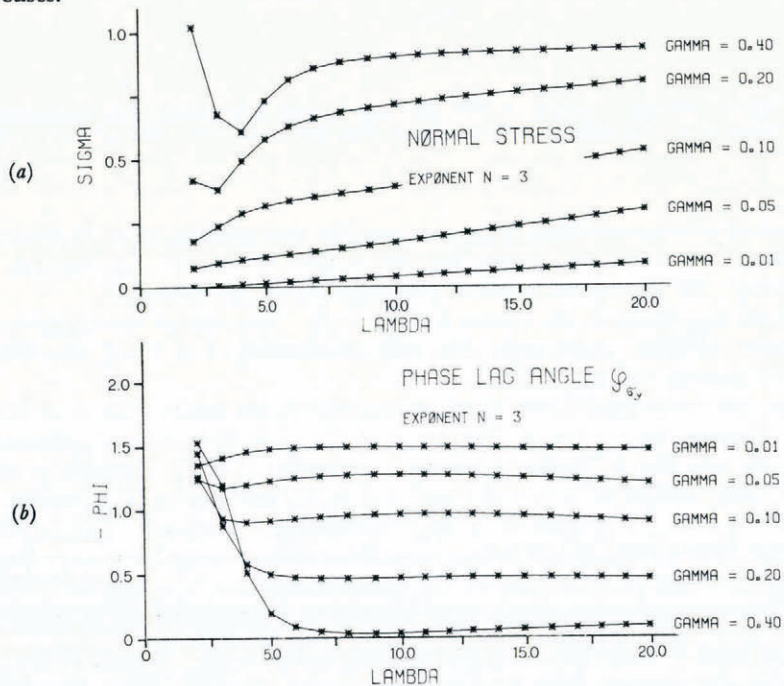


Fig. 9. (a) First-order amplitude of normal stress Σ_0 plotted against λ and for various values of the inclination angle γ . The exponent in the generalized Glen flow law is $n = 3$ and $k = 10^{-3}$. No slip at the bottom. (b) Phase lag angle ϕ_τ corresponding to Σ_0 plotted against λ for various inclination angles γ . The exponent in the generalized Glen flow law is $n = 3$ and $k = 10^{-3}$. No slip at the bottom.

4.3. Surface velocities

It is interesting to see whether bed undulations could also be seen at the surface when velocities are looked at. Zeroth-order velocities can be evaluated from Equation (37) by setting $y = 1$; they will not be plotted here. First-order velocities can easily be calculated from the stream function ψ by mere differentiation. This allows the representation

$$u_s^{(1)} / u_s^{(0)} = \epsilon U_0 \cos(\omega x - \phi_u), \quad v_s^{(1)} / v_s^{(0)} = \epsilon V_0 \cos(\omega x - \phi_v), \quad (94)$$

where the index s signifies evaluation at the surface and $u_s^{(0)}$ is the zeroth-order surface velocity given in Equation (37) (for $y = 1$). Clearly, first-order velocities will depend on the bottom boundary condition. Here, we present results for *no slip* and postpone corresponding results for sliding to Section 4.5.

Results for $n = 2$ and $n = 3$ do not differ significantly, so that only those for $n = 3$ will be presented. In Figure 10 the first-order longitudinal velocity amplitude U_0 and the corresponding phase lag angle ϕ_u are plotted; generally, the amplitudes U_0 grow with growing wavelength; they are larger, the smaller γ . For small wavelengths they tend to zero. Since $U_0 = O(1)$ for small γ , it is the bottom protuberance amplitude ϵ which indicates whether first-order velocity corrections should be accounted for. The phase lag angle ϕ_u undulates, being positive at moderate wavelengths and negative for larger ones. For small γ , ϕ_u is nearly zero, so that bottom perturbations and longitudinal surface velocities are nearly in phase in this case. For steep glaciers this, however, is no longer so.

The first-order surface velocity component in the y direction is plotted in Figure 11. Accordingly there exists a distinct wavelength λ (which depends on mean inclination) for

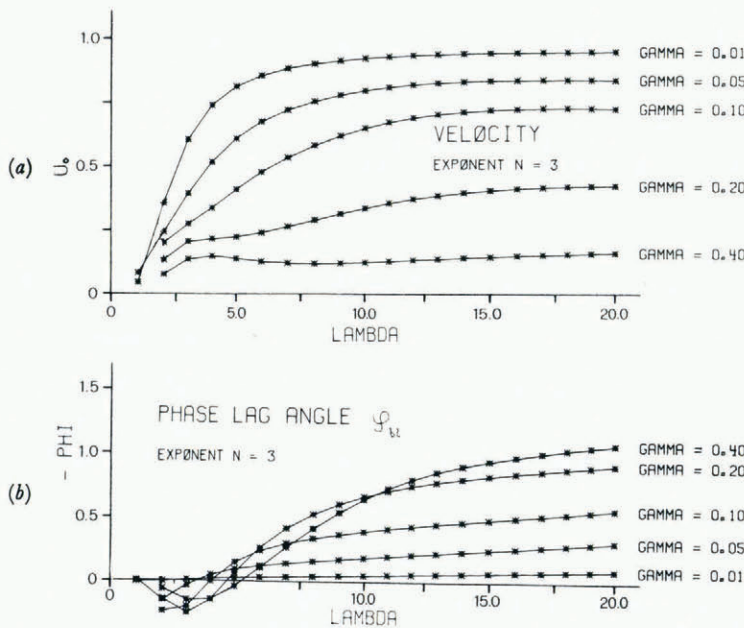


Fig. 10. (a) First-order amplitude of longitudinal velocity U_0 as a function of wavelength λ and plotted for various inclination angles γ . The exponent in Glen's flow law is $n = 3$ and $k = 10^{-3}$. No slip at the bottom. (b) Phase lag angle ϕ_u corresponding to U_0 as a function of λ , parameterized for various inclinations γ . The exponent in Glen's flow law is $n = 3$ and $k = 10^{-3}$. No slip at the bottom.

which V_0 is maximized; moreover, V_0 is $O(1)$ for large γ and becomes negligibly small for small γ . The phase lag angle ϕ_v is shown in Figure 11b. For large λ it is nearly independent of wavelength, but shows a clear dependence on γ . For small γ , bottom undulations and surface velocity components are nearly out of phase.

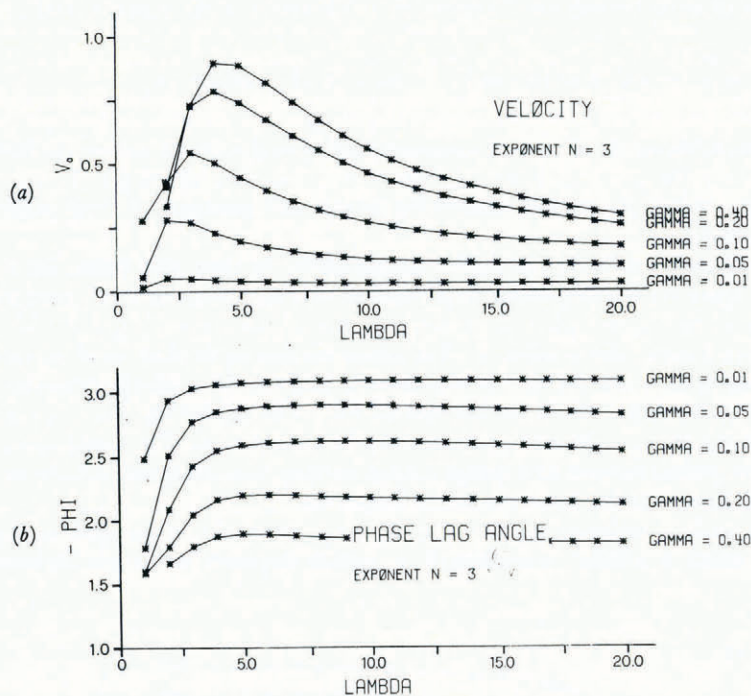


Fig. 11. (a) First-order amplitude of normal surface velocity V_0 as a function of λ and γ . The constants in Glen's flow law are $n = 3$ and $k = 10^{-3}$. No slip at the bottom. (b) Phase lag angle ϕ_v corresponding to V_0 as a function of λ and γ . The exponent in the generalized Glen flow law is $n = 3$ and $k = 10^{-3}$. No slip at the bottom.

4.4. Effect of steady accumulation rate

We showed in Section 3.8 how steady accumulation rate can be accounted for and also indicated that explicit solutions for a Navier–Stokes fluid were constructed. Calculations were performed for a sinusoidal variation of the accumulation rate. This could be regarded as one term of a general harmonic analysis. We chose $A = -A_0 \sin(\omega x)$, where $A_0 = w(\bar{u} + u_r)$. This choice was taken, since for this case $Y_s^{(a)} = H \cos(\omega x)$, with $H = 1$. The real amplitude $\text{Re } H$ can then be obtained from real accumulation amplitudes $\text{Re } A$ according to

$$\text{Re } H = \frac{\lambda \text{Re } A}{2\pi(\bar{u} + u_r)}. \quad (95)$$

Dependent upon the value of $\text{Re } A$, wavelength (which is of the order 10^0 – 10^2) and the surface velocity, $\text{Re } H$ may be as large as $O(1)$. Calculations for $H = 1$ thus lead to realistic results. We refrain from presenting all pertinent details, but shear-stress and normal-stress amplitudes Θ_0 and Σ_0 may be presented. They are displayed in Figure 12 and demonstrate that the effect of accumulation rate on basal stresses is of the same order as that due to bottom

protuberances (compare Figures 8 and 9a). *Neglect of accumulation rate in the evaluation of longitudinal strain effects is therefore not in general permissible.* Incidentally, this same inference can be drawn also from the longitudinal velocity amplitudes, which for $H = 1$ are even larger than those for bottom undulations.

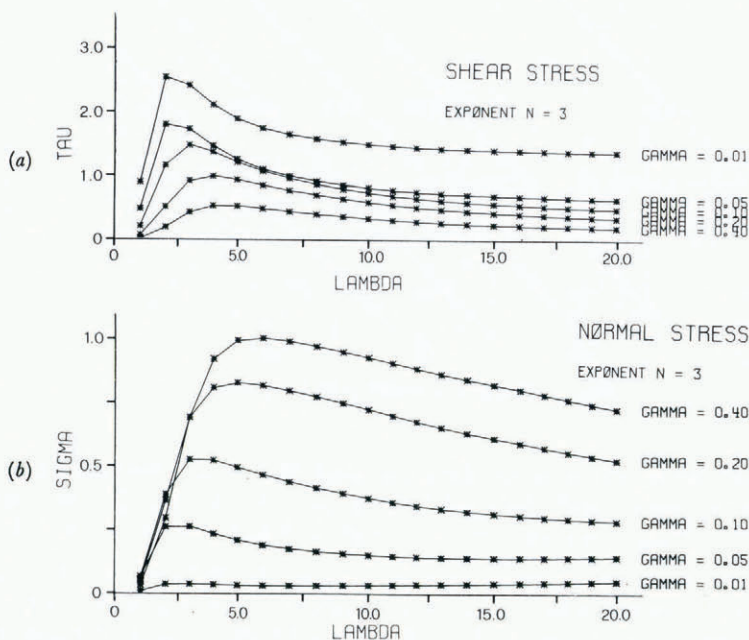


Fig. 12. (a) First-order amplitude of shear stress Θ_0 for sinusoidal steady-state accumulation rate, $A = -\omega u_t \sin(\omega x)$, as a function of λ and γ . Parameters in the generalized Glen flow law are $n = 3$, $k = 10^{-3}$. No slip at the bottom. (b) First-order amplitude of normal stress Σ_0 for sinusoidal steady-state accumulation rate, $A = -\omega u_t \sin(\omega x)$, as a function of λ and γ . Parameters in the generalized Glen flow law are $n = 3$ and $k = 10^{-3}$. No slip at the bottom.

4.5. The effect of sliding at the bottom

All previous numerical results have been presented using the no-slip condition at the bottom. The general theory has, however, been developed for two different sliding conditions, namely that

- (i) a sliding velocity \bar{u} is prescribed and
- (ii) a Weertman-type sliding law, $u_{||} = \mathcal{C} \tau_{||}^m$, is used.

There are at least two good reasons for studying both these cases. First, zeroth-order solutions are not different if we simply relate \bar{u} and \mathcal{C} according to $\bar{u} = \mathcal{C} \sin^m \gamma$, where $m = 2$. Differences in the form of the sliding condition only occur in the first-order correction problem. Since longitudinal strain effects manifest themselves at this level, it is interesting to see to what extent these boundary effects can be seen in the transfer function of bottom undulations and in the basal stresses. This brings us to the second reason. As is well known, Weertman's sliding law is not universally accepted as the proper one. A sensitivity analysis of the solution to both the above sliding laws is therefore worthwhile.

In the calculations described below, \bar{u} has been varied, taking values between 10^{-5} and 10^{-3} . (If $\tau = 5$ d, $D = 500$ m, $\bar{u} = 10^{-3}$ then the sliding velocity is 1 m/d. $\bar{u} = 10^{-3}$ may

thus be regarded as a reasonable non-surging upper bound.) For each inclination angle γ and for $m = 2$, the corresponding value of \mathcal{C} was calculated using $\bar{u} = \mathcal{C} \sin^m \gamma$; for ease of reference, this relation is shown in Figure 13.

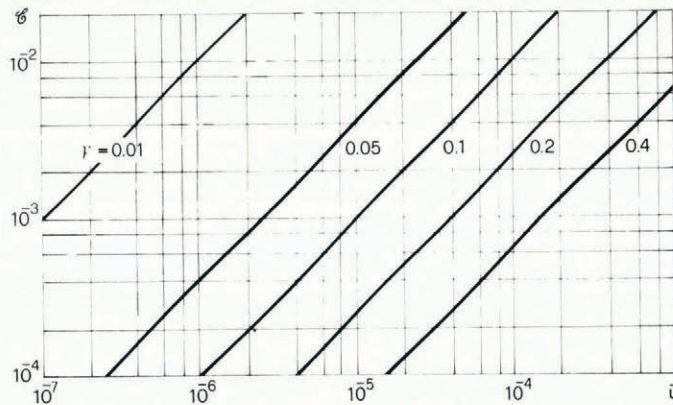


Fig. 13. Plot of $\bar{u} = \mathcal{C} \sin^m \gamma$ for $m = 2$.

Before we present the results, it should be mentioned that, as was the case for no slip, the perturbation scheme applied will fail for small wavelengths. For no slip, the lower bound was roughly $\lambda = 1$; with sliding it increases and is as large as $\lambda \approx 5$ when $\bar{u} = 10^{-3}$. The reason for this behaviour is a rapid growth of $\sigma_y^{(1)}$ as λ becomes small. Failure is not numerical as a similar behaviour is also observed in the analytical solution for the Navier–Stokes fluid. (It is, however, not surprising that the finite-difference scheme reacts critically in these instances.)

4.5.1. Transfer of bottom topography to surface

Filter functions for the transfer of bottom undulations to the surface were plotted for both the sliding conditions mentioned above and for five different values of the sliding velocity \bar{u} in the range $10^{-5} \leq \bar{u} \leq 5 \times 10^{-3}$. For case (i) (\bar{u} prescribed at the bottom) filter functions do not change in character, when compared with those for no slip. Transfer of bottom undulations to the surface is very small for small wavelengths and increases with increasing λ . In particular, no distinct wavelength exists for which transfer would be maximized. Moreover, with increasing \bar{u} the dependence of the filter function on γ becomes less and less. Corroboration for this is provided by Figure 14, in which at most the onset of an intermediate maximum of the filter function can be observed, and this only for specific values of γ . For small γ (< 0.01) and for $\lambda < 2$ these results must not be taken for granted, since first-order stresses become large in this case and perturbation solutions must fail.

The corresponding behaviour for the Weertman boundary condition is quite different! At small \bar{u} ($= 10^{-5}, 10^{-4}$) filter functions show the same characteristic behaviour as they do for no slip. Maximum transfer occurs at very large wavelengths. At $\bar{u} = (5 \times 10^{-4}, 10^{-3})$ and for small mean inclinations $\gamma \leq 0.05$ a clear maximum exists between $3 < \lambda < 5$, which is akin to the result obtained by Budd in an analysis we claimed to be invalid, see Figure 15. Our results seem to give the correct answer to the original problem tackled by Budd, but unlike Budd, who claims that a distinct wavelength for optimal transfer of undulations exists independent of the sliding law, we conclude that in order for this optimum to exist, sliding velocities must be right; if they are too low no clear maximum develops; if they are too high ($\bar{u} = 5 \times 10^{-3}$, for instance, not graphically displayed here) such a maximum has disappeared

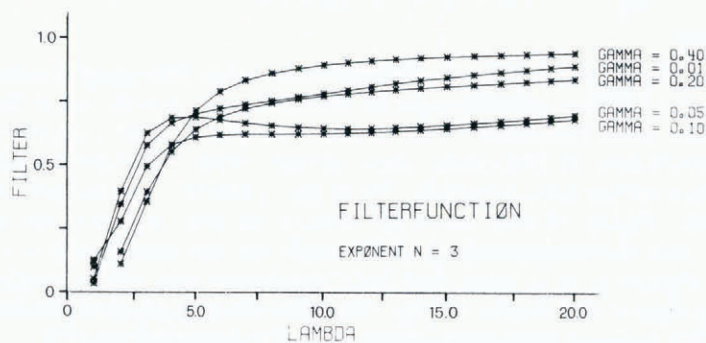


Fig. 14. Filter function for the transfer of bottom undulations. Dimensionless sliding velocity at the bottom is $\bar{u} = 5 \times 10^{-4}$ and the parameters in the generalized Glen flow law are $n = 3$ and $k = 10^{-3}$.

again. At $\bar{u} = 5 \times 10^{-4}$ it exists for $\gamma = (0.01, 0.05)$.* With $D = 1000$ m and $\tau = 1$ d the corresponding basal sliding velocity would be 0.5 m/d. It thus appears that the cause of the predominant transfer frequency is Weertman-type sliding paired with appropriate mean inclinations. This must be so at least for glaciers which are temperate throughout. Whether this behaviour persists also for cold ice sheets has yet to be seen.

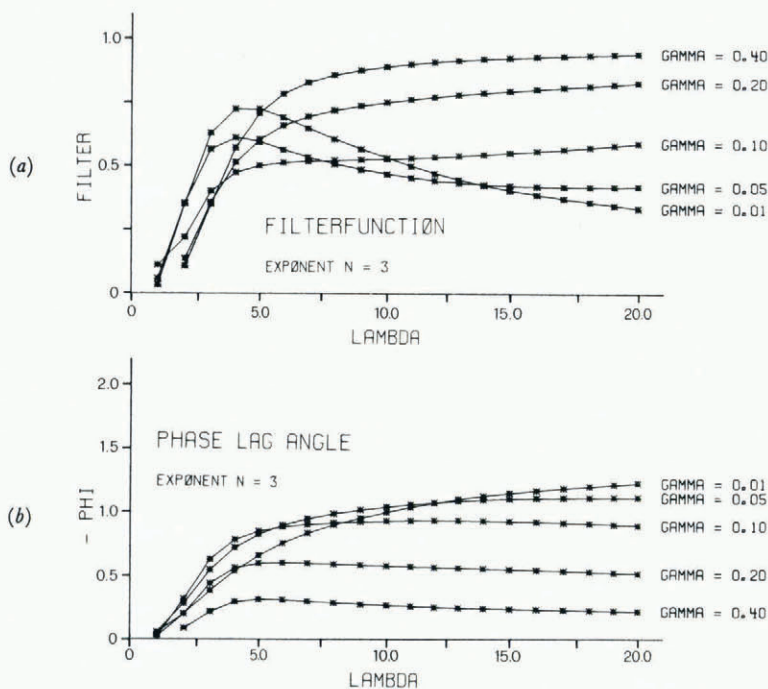


Fig. 15. (a) Filter function \mathcal{F} plotted against wavelength λ for various inclination angles γ . The dimensionless sliding velocity is $\bar{u} = 5 \times 10^{-4}$ and the parameters in Glen's flow law are $n = 3$ and $k = 10^{-3}$. (b) Phase lag angle $\varphi_{\mathcal{F}}$ corresponding to \mathcal{F} as a function of λ and γ . The dimensionless sliding velocity is $\bar{u} = 5 \times 10^{-4}$ and the parameters in the Glen flow law are $n = 3$ and $k = 10^{-3}$.

* The maximum for the curve $\gamma = 0.01$ must be regarded with some caution because for $3 < \lambda < 5$ shear stresses are high, which requires ϵ to be sufficiently small for the perturbation scheme to remain valid.

4.5.2. Basal stresses

Differences in sliding conditions are also detectable, when first-order stresses are looked at. For the same range of sliding velocities and for both boundary conditions at the base the first-order basal stresses were evaluated. The results may, perhaps, be summarized as follows: For boundary condition (i) first-order shear stresses react critically to sliding velocities, provided that mean inclinations are small. For larger values of γ , the behaviour is very similar to that for no slip. Figure 16 may serve as evidence for this. It shows the first-order shear stress amplitude and the corresponding phase angle for the case that $\bar{u} = 5 \times 10^{-4}$. When compared with Figure 7, it is seen that both amplitude and phase angle depend more significantly on γ than is the case for no slip. The perturbation solution becomes questionable for $\gamma = 0.01$. This becomes even more pronounced for larger values of \bar{u} .

It is quite different for the Weertman-type boundary condition (ii)! Here sliding seems to "stabilize" the perturbation scheme, as first-order corrections decrease in size with increasing sliding velocity. As an example we show the first-order shear stresses for $\bar{u} = 5 \times 10^{-3}$ (Fig. 17). For wavelengths $\lambda > 1$ shear-stress amplitudes are all $O(1)$ irrespective of the mean inclination angle. Corresponding phase angles are very small. For smaller sliding velocities the behaviour

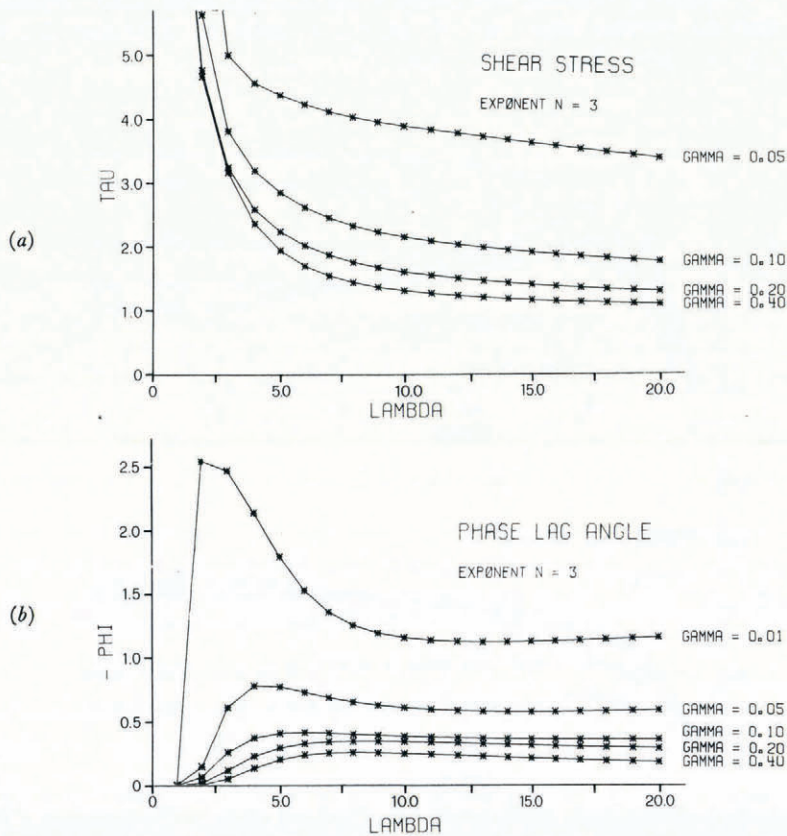


Fig. 16. (a) First-order amplitude of shear stress Θ_0 as a function of λ and γ . Note that results for $\gamma = 0.01$ are outside the range of the plot. Parameters in the generalized Glen flow law have the values $n = 3$, $k = 10^{-3}$ and sliding velocity is prescribed as $\bar{u} = 5 \times 10^{-4}$. Slip according to condition (i). (b) Phase lag angle ϕ_τ corresponding to Figure 16(a). Notice that for $\gamma = 0.01$ ϕ_τ varies appreciably with λ and γ . Perturbation solution must fail in this case. Conditions are otherwise the same as for Figure 16(a).

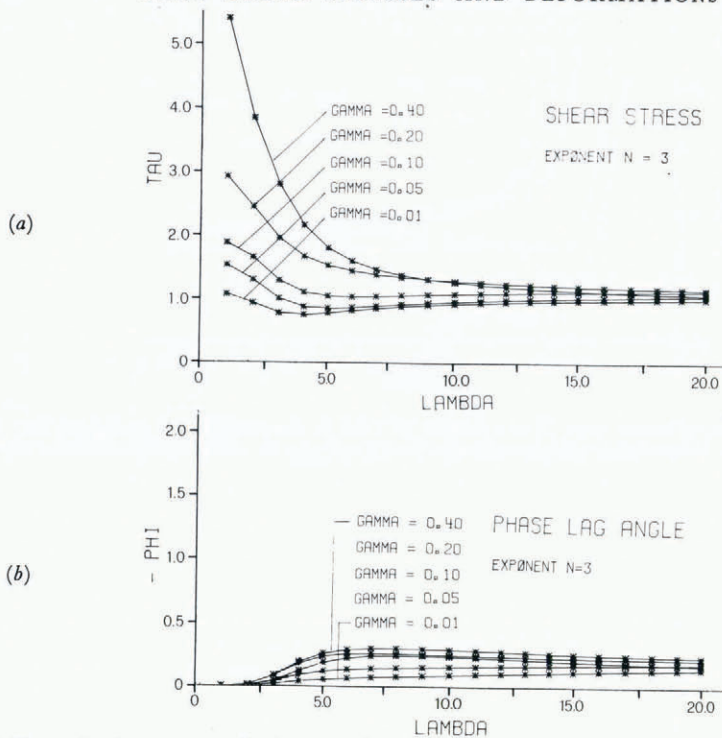


Fig. 17. (a) First-order shear stress amplitude Θ_0 as a function of λ and γ . Notice the regular behaviour for all $\lambda > 1$ and all γ . Parameters for Glen's flow law are $n = 3$, $k = 10^{-3}$. Dimensionless sliding velocity is $\bar{u} = 5 \times 10^{-3}$, but the sliding condition is that due to Weertman. (b) Phase lag angle φ_T corresponding to Figure 17(a). The choice of the parameters and boundary condition is the same as that for Figure 17(a).

is similar with less pronounced independence of the shear stresses on γ (compare with Figure 7) and with an increasingly singular behaviour at small wavelengths, $\lambda < 2$ say.

As far as first-order normal stresses σ_y are concerned, results are reasonably insensitive to differences in boundary conditions ((i) versus (ii)), but the dependence on sliding conditions is pronounced. As was shown in Figure 9 for no slip, first-order normal stresses depend critically on γ ; when γ is large and λ is small, Σ_0 becomes large. With growing sliding velocity \bar{u} , the dependence on γ becomes smaller and smaller, but the singular behaviour at moderately small values of λ is also carried over to small inclinations γ . As an example, we show in Figure 18 the first-order normal stress amplitude Σ_0 for the Weertman-type boundary condition and a sliding velocity $\bar{u} = 5 \times 10^{-3}$. As can clearly be seen, the perturbation scheme must fail at moderately small wavelengths at all indicated mean inclination angles. At smaller sliding velocities the behaviour lies between those shown in Figures 9a and 18.

We conclude by stating that first-order basal shear stresses must depend critically on the sliding law, in particular at small angles γ . Of the two sliding laws, that due to Weertman (ii) leads generally to smaller first-order stresses at comparable sliding velocities. It implies that the perturbation analysis shown here remains valid for a wider range of wavelengths (down to $\lambda = 1$) and inclination angles γ than is the case for the other boundary condition (i). Of course, condition (ii) is more reasonable than (i), but this is no proof of the suitability of the former. Our analysis rather indicates that first-order shear stresses in a temperate ice sheet may depend critically on the basal boundary condition. As long as experts debate the superiority of one over the other, it seems therefore illusory to estimate longitudinal strain effects in temperate glaciers.

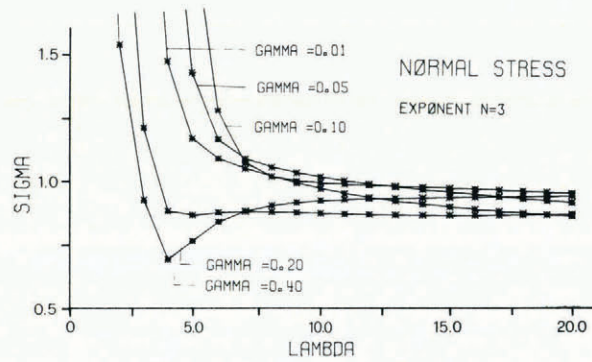


Fig. 18. First-order amplitude of normal stress Σ_0 as a function of λ and γ . Parameters in the generalized Glen flow law are $n = 3$ and $k = 10^{-3}$. Dimensionless sliding velocity is $\bar{u} = 5 \times 10^{-3}$ and the sliding condition is that due to Weertman.

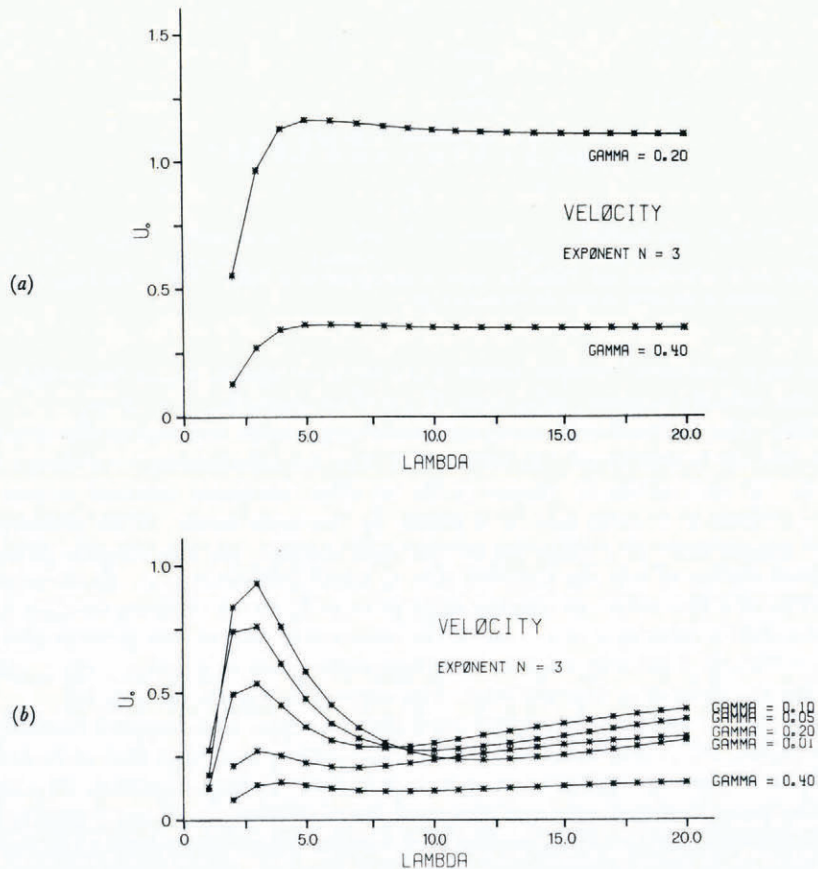


Fig. 19. (a) First-order longitudinal velocity amplitude U_0 as a function of λ and γ . The parameters in the generalized Glen flow law are $n = 3$ and $k = 10^{-3}$. Dimensionless sliding velocity is $\bar{u} = 5 \times 10^{-3}$ and boundary condition (i) is used. (b) First-order amplitude of longitudinal velocity U_0 as a function of λ and γ . The parameters in the generalized Glen flow law are $n = 3$ and $k = 10^{-3}$. Dimensionless sliding velocity is $\bar{u} = 5 \times 10^{-3}$ and the sliding condition is that due to Weertman.

4.5.3. First-order velocities

It is now no longer surprising that differences in the boundary conditions manifest themselves also in the first-order longitudinal velocities. For the boundary conditions (i) the velocity amplitude U_0 depends strongly on the value of \bar{u} ; for the Weertman-type condition (ii) it does not. As an example, the graphs of Figure 19 may serve. They show the first-order longitudinal velocity amplitude U_0 for a value of $\bar{u} = 5 \times 10^{-3}$. In Figure 19a the results for boundary condition (i) are displayed. Only plots for $\gamma = (0.4, 0.2)$ are shown as those for smaller γ lie outside the range of the ordinate. This behaviour is quite unlike that for boundary condition (ii)! Here, U_0 remains $O(1)$ or smaller for all inclination angles $0.01 \leq \gamma \leq 0.4$. There is a smooth transition in this case from the behaviour for no slip (see Fig. 10) to that of Figure 19b, where sliding velocities increase with no indication of unusual behaviour. The distribution of U_0 for boundary condition (i) is completely different. At certain sliding velocities it behaves qualitatively as shown in Figure 19a, at others more like that in Figure 19b.

Interestingly, the vertical surface velocity is insensitive to the form of the boundary condition at the bottom. But sliding does affect the size and the wavelength behaviour of V_0 and also its phase. For corroboration the reader may compare Figures 11a and 20, the latter being valid for a Weertman-type boundary condition and a sliding velocity $u = 5 \times 10^{-3}$.

Many more results could be shown, but space limitations prevent us from doing so.

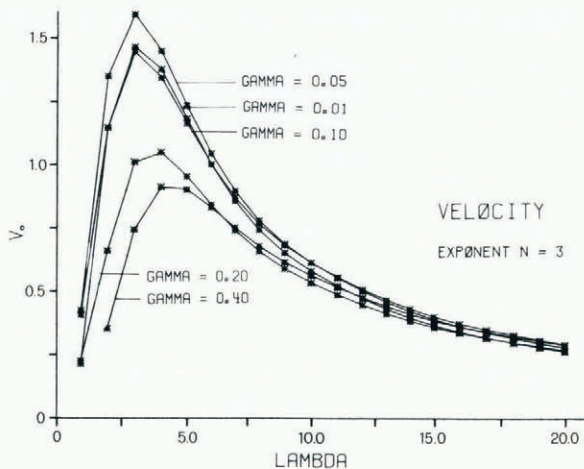


Fig. 20. Amplitude of normal velocity V_0 as a function of λ and γ . Parameters in the generalized Glen flow law are $n = 3$ and $k = 10^{-3}$. Dimensionless sliding velocity is that due to Weertman.

4.6. Temperature variations and the longitudinal strain effects in cold ice

All the foregoing calculations were performed for ice sheets with spatially independent material properties. For temperate glaciers such conditions are satisfied to a sufficient degree of accuracy, but in cold ice sheets such as Greenland and Antarctica, material properties change drastically with depth, because of the appreciable dependence of the Arrhenius factor on temperature. It is interesting to investigate to what extent the above analysis remains correct when temperature variations perpendicular to the main flow direction are taken into account.

First, results for an ice sheet which is cold throughout and therefore adheres to the bed have been obtained by Hutter and Spring (1979). They demonstrate that transfer of bottom

undulations, first-order basal shear and normal stresses, and first-order surface velocities, are all attenuated by vertical temperature variations.

If the ice sheet reaches the pressure-melting point at the base, sliding is possible; the inferences drawn by Hutter and Spring may then change. Our analysis remains valid when material properties change with depth. All that is needed is an adjustment of the form of the function \mathfrak{B} , which may be written as

$$\mathfrak{B}(\tau_{II^2}, y) = \exp \left[-\frac{Q}{k} \left(\frac{1}{T(y)} - \frac{1}{T_0} \right) \right] \frac{(\tau_{II^2})^{(n-1)/2} + k}{1+k} \quad (96)$$

where Q is the activation energy, k the Boltzmann constant, T_0 the surface temperature, and $T(y)$ the temperature distribution with depth. Calculations were performed for

$$T(y) = \theta_1 \{ 1 + \theta_2 [\operatorname{erf}(L^*) - \operatorname{erf}(L^*y)] \}, \quad (97)$$

in which $\theta_1 = 258.8$ K, $\theta_2 = 1.05$, $L^* = 3.70$, and erf is the error-function. This choice corresponds to an ice sheet 1400 m thick whose bottom boundary is at the melting point (272.12 K) and of which the upper surface is at the temperature 258.8 K; needless to say, Equation (97) is a solution of the heat conduction equation.

In the zeroth-order solution, temperature variations manifest themselves in the longitudinal velocity distribution; the first of Equations (37) remains valid as an expression for $u^{(0)}(y)$, but integrations must now be carried out numerically. In the analysis of the first-order velocities (see Equation (94)), the surface velocity u_s is used. Clearly, the numerical analysis of the first-order problem was based on this numerically-determined surface velocity.

Calculations for the first-order problem were performed for sliding at the bottom according to both sliding laws (i) and (ii), and \bar{u} was given the values 10^{-4} and 10^{-3} . The results may, perhaps be summarized as follows:

- (1) As regards the transfer of bottom undulations, filter functions are reduced in general by the vertical temperature variation. This is uniformly so for large wavelengths, but not necessarily so for smaller ones. Moreover, the effect of maximum transfer of bottom protuberances to the surface, which has been observed for some angles (see Figure 14), may be amplified by temperature variations. Corroboration is given by Figure 21, which shows the filter function for $\bar{u} = 10^{-3}$ and Weertman-type sliding (maxima for sliding law (i) are less pronounced).

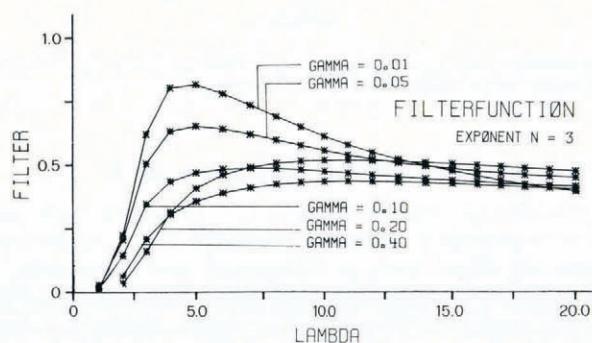


Fig. 21. Filter function \mathcal{F} for a cold ice sheet plotted against λ for various values of γ . The temperature distribution was according to Equation (97) and the parameters in Glen's flow law are $n = 3$, $k = 10^{-3}$. The boundary condition at the bed was that due to Weertman and $\bar{u} = 10^{-3}$.

- (2) First-order shear-stress amplitudes are also reduced by the inclusion of temperature variations. This is uniformly so for all investigated sliding velocities and for either sliding law. As an example, we show in Figure 22 the shear-stress amplitudes Θ_0 as obtained when the temperature distribution given by Equation (97) and a Weertman-type boundary condition were used. The dimensionless sliding velocity was $\bar{u} = 10^{-4}$. For comparison, results are also shown for a temperate glacier and an ice sheet whose material properties are kept constant and which adheres to the bed. Figure 22 is sufficient proof of the importance of both sliding and temperature variation.

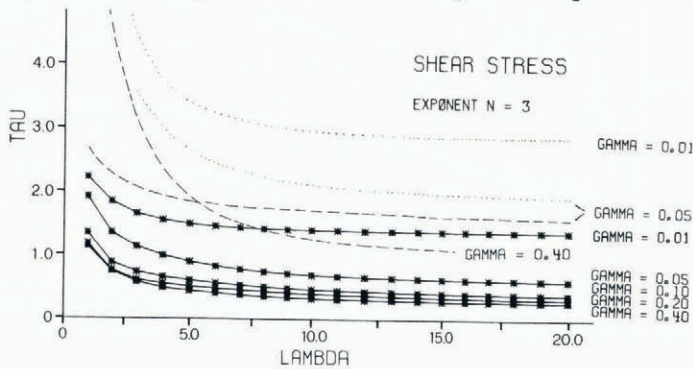


Fig. 22. First-order amplitude of shear stress as a function of λ and γ . The temperature distribution was according to Equation (97) and the parameters in Glen's flow law were $n = 3$ and $\bar{u} = 10^{-4}$. The boundary condition is that of Weertman with a sliding velocity $\bar{u} = 10^{-4}$. Also shown are results for an ice sheet in which temperatures were kept constant (dashed) and, in addition, the no-slip condition (pointed) was used.

- (3) Equally interesting are results regarding first-order normal stresses Σ_0 . Whereas sliding (as opposed to no slip) has not led to a reduction in the first-order normal stresses (see Figs 9a and 18), the inclusion of temperature variations brings a reduction, especially at wavelengths $1 < \lambda < 5$, at large mean inclinations γ , and at small sliding velocities. For no slip it seems to be largest. Figure 23 may serve as an example, it shows results for $\bar{u} = 10^{-4}$ and the temperature distribution of Equation (97), and a comparison for constant temperature.
- (4) First-order velocities U_0 and V_0 are also changed considerably by the inclusion of vertical temperature variations. For $\gamma > 0.1$ these changes seem to be insignificant, but for smaller values of the mean inclination γ the changes are substantial. Figure 24 may serve as an example.

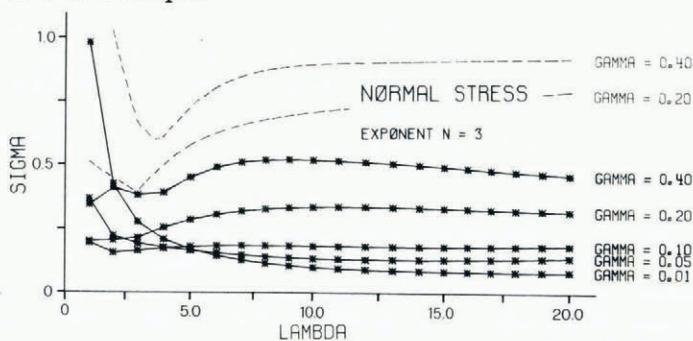


Fig. 23. First-order amplitude of normal stress as a function of λ and γ . Shown are results for $\bar{u} = 10^{-4}$ and a temperature distribution according to Equation (97). For comparison results are also shown for a constant temperature distribution. The parameters in Glen's flow law are $n = 3$ and $\bar{u} = 10^{-4}$, and Weertman-type sliding was used.

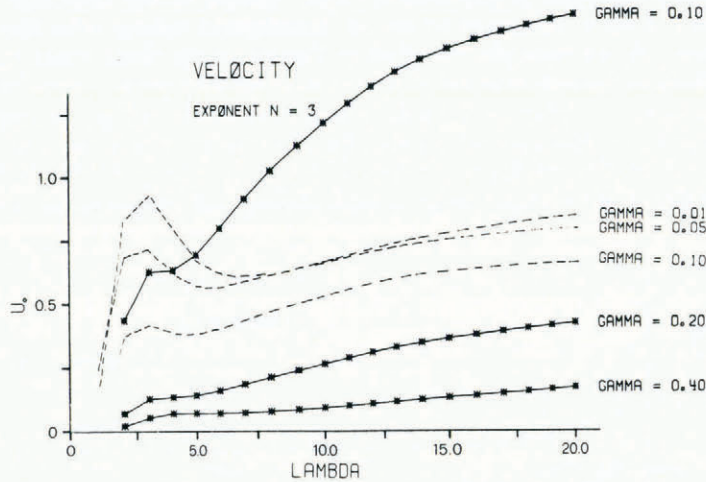


Fig. 24. First-order amplitude of longitudinal velocity as a function of λ and γ . Results are shown for $\bar{u} = 10^{-3}$ and a temperature distribution according to Equation (97). For comparison results are also shown for a constant temperature distribution. The parameters in Glen's flow law are $n = 3$, $k = 10^{-3}$, and Weertman-type sliding was used.

5. CONCLUDING REMARKS

In this article, first-order stresses and deformations in an ice slope have been determined. The fundamental governing equations have been formulated on the basis of the continuum mechanics of slow viscous flow of non-linear fluids. In the solutions, efficient use has been made of perturbation approximations, first to separate steady-state from time-dependent problems, and, secondly, by assuming that bottom undulations deviate only slightly from a plane. Using the amplitude of the bottom undulations essentially as the perturbation parameter, first-order corrections of the velocities and stresses could then be determined from a linear boundary-value problem. A detailed analysis of the steady-state flow of a nearly parallel-sided ice slope has been given, and emphasis laid on the transfer of bottom undulations to the surface, on longitudinal strain effects on basal stresses and surface velocities, and on the effect of spatial variations of these. Results can be summarized as follows.

Contrary to previous claims, there is no universally preferred wavelength at which transfer of bottom protuberances to the surface is optimal. Generally, this transfer increases with increasing wavelength, yet at very distinct mean inclination angles and distinct values of the sliding velocity, filter functions do show a preferred wavelength at which transfer of bottom undulations to the surface is maximal. These wavelengths lie between $3 < \lambda < 5$, the maxima appear to be more pronounced when vertical temperature variations are taken into account than when constant material properties are assumed throughout. (We do not know whether mean inclinations and sliding velocities in the instances when preferred wavelengths were observed in the field were such that a preferred wavelength transition to the surface does exist.) The differences between our theoretical findings and those of Budd are important as they indicate that the wavelength spectra of steady-state surface undulations may not possess a distinct wavelength the cause of which could uniquely be related to bottom protuberances. If sliding and inclination are right, they do, otherwise one might speculate that x -dependent zeroth-order solutions, of the kind which must occur below an ice fall, might provide the answer to it (see Williams, 1979).

As far as basal stresses are concerned, the analysis has indicated that their order of magnitude strongly depends on the spatial temperature distribution and on the type of sliding

law. Longitudinal strain effects on basal stresses are roughly twice as large in temperate ice as they are in cold ice. Moreover, for a Weertman-type sliding law they are smaller than for a sliding law in which sliding velocities are prescribed. This result is particularly important, as it indicates that the first-order stress distribution depends critically on the sliding law. Therefore as long as experts debate the proper form of the sliding law it is illusory to estimate longitudinal strain effects on basal stress.

Differences due to basal boundary conditions are even more pronounced in the surface velocities than they are in the basal stresses. For Weertman-type sliding, first-order surface velocities are generally smaller than when sliding velocities are prescribed.

Analysis of the accumulation-rate effects on the basal stress distribution showed further that it is in general not justified to neglect their effect.

The above results are all subject to the validity of the applied perturbation scheme. Results indicate that this scheme breaks down at small wavelengths; the lower bound of wavelength to which solutions remain valid depends on the inclination of the ice slab and the sliding law. Typically, wavelengths must not be smaller than the ice thickness and for small inclinations of the ice slab may have to be as large as three times the ice thickness.

As a consequence of these facts, calculations of the effects of longitudinal strain on basal shear as performed by Budd (1971) are questionable, and conclusions from them must most likely be invalid. We could, if we so desired, repeat the Budd's analysis with our improved approach, but shall not do it here for the following reason. Bottom and top surfaces were assumed to deviate only slightly from straight parallel lines and protuberance amplitudes were assumed to be small compared with the glacier thickness. Such assumptions are somewhat unrealistic; top and bottom surface are rarely parallel, but their change along the axis is usually slowly varying. Stress variations with ice flow over undulations is thus better analysed using this less stringent hypothesis. Hutter (1981) presents the pertinent details.

The analysis also showed that the temperature variation, which manifests itself in a position-dependent stress-strain relationship, may be taken into account without further difficulties. Contrary to the usual understanding, however, the ice slope need not be divided into a series of strips of uniform properties and, in particular, no restriction to a linear viscous law is required. Material properties may vary continuously according to independently-taken temperature records. Owing to space limitations, results have only been sketched in this regard. A detailed analysis for no slip has been given by Hutter and Spring (1979), and results for sliding at the bed have only been demonstrated for a very limited range of applications. They showed that under no circumstances is it allowable to disregard temperature variations in cold ice, and in particular, it is shown that filter functions and basal stresses are reduced by temperature variations.

The above discussion is restricted to steady-state problems, but the theory was presented initially also for time-dependent problems. In particular, it would be interesting to see whether a kinematic wave theory could be derived from our governing equations. Formally this is the case; in fact Hutter (1980) gives the corresponding analysis. Explicitly he shows that the governing equations of surface waves travelling down an inclined slope can be derived from the above set of equations by merely invoking a long wavelength. To a first approximation, kinematic wave theory does evolve from the corresponding analysis, but calculations indicate it to be oversimplified. For details the reader is referred to Hutter (1980).

Still further extensions are possible, however. The influence of global curvature effects is one of these, the behaviour at small wavelengths would be another. Future investigations will have to deal with these as well as different sliding laws and cavity formation.

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